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# **ESR-1539**

**ICC-ES Evaluation Report** 

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Reissued 07/2018 Revised 06/2019 This report is subject to renewal 07/2020.

DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES SECTION: 06 05 23.13—NAILS SECTION: 06 05 23.15—STAPLES

**REPORT HOLDER:** 

# **INTERNATIONAL STAPLE, NAIL AND TOOL ASSOCIATION (ISANTA)**

**EVALUATION SUBJECT:** 

# **POWER-DRIVEN STAPLES AND NAILS**



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## **ICC-ES Evaluation Report**

#### **ESR-1539**

Reissued July 2018 Revised June 2019 This report is subject to renewal July 2020.

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DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES Section: 06 05 23.13—Nails Section: 06 05 23.15—Staples

**REPORT HOLDER:** 

INTERNATIONAL STAPLE, NAIL AND TOOL ASSOCIATION (ISANTA)

**ADDITIONAL LISTEES:** 

AMERICAN FASTENERS CO. LTD.

**BECK AMERICA, INC.** 

**BUILDING MATERIAL DISTRIBUTORS, INC.** 

FALCON FASTENERS REG'D

HUTTIG BUILDING PRODUCTS

JAACO CORPORATION

KOKI HOLDINGS AMERICA LTD.

KYOCERA SENCO INDUSTRIAL TOOLS, INC.

MID-CONTINENT STEEL & WIRE (MID-CONTINENT NAIL)

OMAN FASTENERS, LLC.

PASLODE, AN ILLINOIS TOOL WORKS COMPANY

PEACE INDUSTRIES

PRIMESOURCE BUILDING PRODUCTS

SPECIALTY FASTENING SYSTEMS, INC.

STANLEY BLACK AND DECKER INC.

**EVALUATION SUBJECT:** 

POWER-DRIVEN STAPLES AND NAILS

1.0 EVALUATION SCOPE

Compliance with the following codes:

A Subsidiary of the International Code Council<sup>®</sup>

- 2018, 2015, 2012, 2009 and 2006 International Building Code<sup>®</sup> (IBC)
- 2018, 2015, 2012, 2009 and 2006 *International Residential Code*<sup>®</sup> (IRC)

For evaluation for compliance with codes adopted by the Los Angeles Department of Building and Safety (LADBS), see the <u>ESR-1539 LABC and LARC Supplement</u>.

#### **Properties evaluated:**

- Bending yield strength
- Compliance with prescriptive requirements of the IBC and IRC.
- Compliance with material requirements, dimensions and tolerances of ASTM F1667.
- Use in diaphragms, shear walls and braced walls.
- Fastening schedules which are alternates to those included in the codes.

#### 2.0 USES

The nails and staples described in this report are used for engineered and nonengineered (prescriptive) structural connections.

#### 3.0 DESCRIPTION

#### 3.1 General:

The fasteners recognized in this report are manufactured by and for the additional listees on this report, which are member companies of the International Staple, Nail and Tool Association (ISANTA). Appendix B of this report lists the fasteners recognized for each listee.

#### 3.2 Staples:

Recognized staples are manufactured from bright or zinccoated carbon steel wire. Recognized staples comply with Table 60 of ASTM F1667-17 and have the characteristics shown in the table below. The staples have a minimum crown width of  $^{7}/_{16}$  inch (11.1 mm) and a minimum leg length of  $1^{1}/_{2}$  inches (38 mm). The staples are collated into strips and cohered with polymer coatings. Staple crown widths and leg lengths specified in this report are overall dimensions.

#### TABLE 3.2—STAPLE CHARACTERISTICS

NOMINAL WIRE DIAMETER (inch)	NOMINAL STAPLE WIDTH (inch)	MINIMUM BENDING MOMENT (lbfin.)
0.080	0.0855	4.3
0.0720	0.073	4.0
0.0625	0.064	3.6
	DIAMETER (inch) 0.080 0.0720	DIAMETER (inch)         STAPLE WIDTH (inch)           0.080         0.0855           0.0720         0.073

For **SI:** 1 inch = 25.4 mm; 1 lbf-in = 0.113 N-m.

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#### 3.3 Nails:

Recognized nails are manufactured from bright steel wire, galvanized steel wire, or stainless steel wire. The nails have full round heads or modified round heads, such as offset heads, clipped heads ("D" heads) and notched heads, as shown in Figure 1. Nails have smooth or deformed (threaded) shanks. Deformed shanks may be annularly threaded (ring shank) or helically threaded (screw shank). Dimensional tolerances conform to ASTM F1667.

Nails designated as Metal Hardware Nails (MHN) are primarily intended for use with metal hardware (e.g. joist hangers, strap anchors, etc.), but may also be used in other engineered and prescriptive wood-to-wood or metalto-wood connections. They have full round heads and smooth or ring shanks.

Nails with coating designated as EG are electrogalvanized in accordance with ASTM A641, Class 1. Nails with coating designated as HDG are hot-dip galvanized and comply with the coating thickness requirement of ASTM A153, Class D. Both EG and HDG nails comply with the requirements of Section 10.1 of ASTM F1667.

Specific nail products are coated with proprietary polymer coatings as indicated in Appendix B. These coatings are intended to aid in the driving of nails when used with power tools.

Nails are collated and cohered into strips or coils for loading into a power driving tool. Typical recognized products are illustrated in Figure 1. Table 1 lists nail sizes addressed in this report. See Appendix B for detailed nail descriptions including bending yield strength for products recognized for each listee. Nails for each listee having the same diameter, shank type and finish type as those listed in Appendix B, are recognized for any length.

#### 3.4 Wood:

Wood members must be as described in the tables in this report. Sawn lumber must have an assigned specific gravity (SG) equal to or greater than what is required in the applicable table. Where use of engineered wood products is addressed in tables in this report, the products must have an equivalent specific gravity (ESG) equal to or greater than the SG that is addressed in the table, as shown in the applicable ICC-ES evaluation report for the engineered wood product.

#### 3.5 Steel Side Plates:

Steel side plates must comply with ASTM A653 SS Grade 33 or 40, or with ASTM A36, as indicated in Table 3B. The steel must have a minimum base steel thickness as indicated in Table 3B. Holes in steel side plates must be predrilled or prepunched to allow for the installation of the nails.

#### 4.0 DESIGN AND INSTALLATION

#### 4.1 Design for Staples:

**4.1.1 Engineered Connections:** Reference withdrawal design values for staples recognized in this report may be calculated in accordance with Section A2.3 of Appendix A. Reference withdrawal design values for select connections are shown in Table 4. The reference lateral design values for staples recognized in this report may be calculated in accordance with Section A2.2 of Appendix A.

4.1.2 Engineered Diaphragms and Shear Walls: The staples recognized in this report may be used in

engineered diaphragms and shear walls, in accordance with the diaphragm and shear wall design tables in the IBC and Tables 5 through 9, when the staples comply with the requirements in the applicable table for gage, crown width and leg length. Diaphragm and shear wall deflection must be determined in accordance with Section A3.2.

**4.1.3 Prescriptive Sheathing Attachments:** The staples recognized in this report may be used to attach sheathing to wood framing as prescribed in the code tables referenced in Table 2, when the staples comply with the code requirements for gage, crown width and leg length.

#### 4.2 Design for Nails:

**4.2.1 Engineered Connections:** All reference design values must be multiplied by all applicable adjustment factors in accordance with the ANSI/AWC National Design Specification for Wood Construction (NDS).

**4.2.1.1 Reference Lateral Design Values:** The nails recognized in this report comply with the requirements of IBC Section 2303.6 and may be used in lateral connections designed in accordance with the NDS, using the specified minimum bending yield strength and the nominal diameter shown in Appendix B, as applicable. The yield mode equations in the NDS for nails are shown in Section A1.2 of Appendix A to this report. Reference lateral design values for common wood-to-wood connections are shown in Table 3A, and reference lateral design values for common metal-side-plate-to-wood connections are shown in Table 3B.

**4.2.1.2 Reference Withdrawal Design Values:** The nails recognized in this report may be used in tension connections designed in accordance with the NDS, using the nominal diameter shown in Appendix B, as applicable, and the embedded length of the nail in the holding member. For stainless steel nails, the reference withdrawal design values must be determined in accordance with the 2018 NDS, for use under the 2018, 2015, 2012, 2009 and 2006 IBC. Reference withdrawal design values for common wood specific gravities are shown in Table 4. The withdrawal equations in the 2018 NDS for nails are shown in Section A1.3 of Appendix A to this report.

**4.2.1.3 Reference Head Pull-through Design Values:** For nails shown in Appendix B as having round heads, reference head pull-through values must be determined in accordance with Section 12.2.5 of the 2018 NDS, for use under the 2018, 2015, 2012, 2009 and 2006 IBC. For nails shown in Appendix B as having other head styles, determination of reference head pull-through design values is outside the scope of this report.

**4.2.2 Prescriptive Framing Connections:** The carbon steel nails may be used for prescriptive framing connections when the nails comply with the requirements in the applicable code for diameter and length. In addition, Tables 10, 11 and 12 show fastening designs for framing connections under the 2018, 2015 and 2012 IBC and IRC, which are alternatives to what is prescribed in 2018 and 2015 IBC Table 2304.10.1 (2012 IBC Table 2304.9.1) and in IRC Table R602.3(1). These alternative fastener designs address the use of stainless steel nails. The alternative fastener designs shown in Tables 10, 11 and 12 are summarized in Table 13.

**4.2.3 Prescriptive Metal Hardware Connections:** Nails designated as Metal Hardware Nails, as well as other nails recognized in this report as having full round heads and the applicable dimensions, may be used to attach metal

hardware (e.g. joist hangers, foundation anchors) to wood framing members as prescribed in ICC-ES evaluation reports on metal hardware. Use of Metal Hardware Nails in diaphragms and shear walls is outside the scope of this report.

**4.2.4 Engineered Diaphragms and Shear Walls:** The nails may be used in shear walls and diaphragms designed in accordance with the ANSI/AWC Special Design Provisions for Wind and Seismic (SDPWS) and the tables in this report when they are of the required material, shank type, diameter and length indicated in Tables 5 through 8 of this report, and when indicated in Appendix B as meeting the head area requirements for use in lateral force resisting assemblies for the applicable nail size.

Allowable shear values for diaphragms comprised of wood structural panels attached to wood framing are shown in Tables 5 and 6. Design of roof diaphragms must consider uplift due to wind.

Allowable shear values for shear walls comprised of wood structural panels attached directly to wood framing or over gypsum sheathing are shown in Tables 7 and 8. Design of exterior shear walls must also consider transverse (out-of-plane) loads on sheathing due to wind.

Allowable shear values for shear walls comprised of fiberboard sheathing, gypsum lath and plaster, gypsum sheathing, gypsum wallboard, metal or wire lath and plaster, or plywood siding applied directly to wood framing are shown in Table 9.

Diaphragm and shear wall deflection must be determined in accordance with Section A3.1.

**4.2.5 Prescriptive Sheathing Attachments:** Table 2 references the code tables where nails are prescribed for attaching sheathing to framing. Carbon steel nails (bright or galvanized) shown in Appendix B as meeting the head area ratio requirements for use in lateral force resisting assemblies may be used where the same nail types and sizes are prescribed in the referenced code tables.

#### 4.3 Installation:

The nails must be installed in accordance with this report, the listee's published installation instructions, the approved plans, if applicable, and the applicable prescriptions in the code.

Nails used with metal hardware (joist hangers, truss plates, etc.) must be installed in accordance with the metal hardware manufacturer's instructions and any applicable ICC-ES evaluation report.

The nails described in this report are packaged for use in power tools. The nails must be installed using a tool recommended by the applicable listee. Individual nails may also be manually driven.

Edge distances, end distances, and spacings must be sufficient to prevent splitting of the wood. Installation into sawn lumber must be in accordance with the applicable requirements of 2018 and 2015 NDS Section 12.1.6 (2012 NDS Section 11.1.6 for the 2012 IBC; 2005 NDS Section 11.1.5 for the 2009 and 2006 IBC).

#### 4.4 Special Inspection:

Periodic special inspection of nailing used in the construction of main windforce-resisting systems is required by 2018 and 2015 IBC Section 1705.11.1 (2012 IBC Section 1705.10.1, 2009 IBC Section 1706.2) when the nail spacing is 4 inches (102 mm) or less. Periodic

special inspection of nailing used in the construction of seismic force-resisting systems is required by 2018 and 2015 IBC Section 1705.12.2 (2012 IBC Section 1705.11.2, 2009 and 2006 IBC Section 1707.3) when the nail spacing is 4 inches (102 mm) or less.

**4.5 Use in Treated Lumber:** In accordance with 2018 and 2015 IBC Section 2304.10.5 (2012, 2009 and 2006 IBC Section 2304.9.5) and IRC Section R317.3 (2006 IRC Section R319.3), stainless steel (SS) and hot-dip galvanized (HDG) nails listed in Appendix B, may be used in preservative-treated and fire-retardant-treated lumber. Use of nails listed in Appendix B as having a proprietary coating for installation in preservative-treated lumber in specific Exposure Conditions, is addressed in Appendix B or in applicable ICC-ES evaluation reports referenced in Appendix B. Nails and staples listed in Appendix B as bright must not be used in treated lumber. Use of nails and staples with other coatings in treated lumber is outside the scope of this report.

#### 5.0 CONDITIONS OF USE

The nails and staples described in this report comply with, or are suitable alternatives to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions:

- **5.1** The nails and staples must be installed in accordance with this report, the listee's published installation instructions, the approved plans (if applicable), and the applicable prescriptions in the code. In the case of a conflict amongst these documents, the most restrictive requirements govern.
- **5.2** The fastener dimensions specified in the design tables in this report are minimum nominal dimensions. When fasteners larger than those specified are used for any application, consideration must be given to restrictions on edge distance and close spacing.
- **5.3** See Section 4.5 regarding use of staples and nails in treated wood.
- **5.4** The nails and staples described in Appendix B of this report are manufactured under quality control programs with inspections by ICC-ES.

#### 6.0 EVIDENCE SUBMITTED

- **6.1** Data in accordance with the ICC-ES Acceptance Criteria for Nails (AC116), dated March 2018.
- **6.2** Data in accordance with the ICC-ES Acceptance Criteria for Staples (AC201), dated March 2018.

#### 7.0 IDENTIFICATION

7.1 Packages of nails and staples must be identified with one of the ISANTA logos shown below or the name of one of the listees identified in this report, the applicable brand name (shown in Appendix B), fastener size (nail diameter and length or staple gage, crown width and length), finish/coating designation, and country of origin. Packages are also identified with the evaluation report number (ESR-1539), ICC-ES mark and the evaluation report number (ESR-1539), "ICC-ES ESR-1539" or an ICC-ES machine readable code which includes the evaluation report number (ESR-1539) as shown below.







ISANTA Logos





ICC- ES Mark

ICC-ES Machine-readable Code **7.2** The report holder's contact information is the following:

INTERNATIONAL STAPLE, NAIL AND TOOL ASSOCIATION 8735 WEST HIGGINS ROAD, SUITE 300 CHICAGO, ILLINOIS 60631 (847) 375-6454 www.isanta.org info@isanta.org

**7.3** The Additional Listees' contact information appears in the Table B1 of Appendix B.

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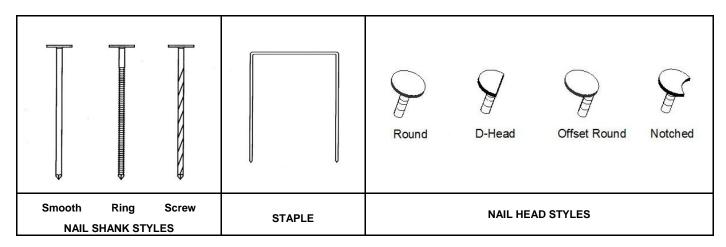


FIGURE 1—BASIC FASTENER STYLES

	DESCR	RIBED IN AS	TM F1667-17		OTHE	RS
SHANK DIAMETER (inch)	TYPE AND PENNYWEIGHT	LENGTH (inches)	HEAD DIAMETER (inch)	SHANK STYLE	COMMONLY AVAILABLE LENGTHS (inches)	SHANK STYLES
0.092	6d cooler	1 <sup>7</sup> / <sub>8</sub>	0.250	Smooth, Ring, Screw	$\begin{array}{c}1^{1}\!/_{4},1^{1}\!/_{2},1^{5}\!/_{8},\\1^{3}\!/_{4},2,2^{1}\!/_{8},\\2^{3}\!/_{16},2^{1}\!/_{4},\\2^{3}\!/_{8},2^{1}\!/_{2}\end{array}$	Smooth, Ring, Screw
0.099	6d box	2	0.266	Smooth	$\begin{array}{c} 1^{1}\!/_{8},1^{1}\!/_{2},1^{3}\!/_{4},\\ 1^{7}\!/_{8},2,2^{1}\!/_{4},\\ 2^{3}\!/_{8} \end{array}$	Smooth, Ring, Screw
	6d common	2	0.266		0.01/.03/	Smooth,
0.113	8d box	2 <sup>1</sup> / <sub>2</sub>	0.297	Smooth	$2, 2^{1}/_{4}, 2^{3}/_{8}, 2^{1}/_{2}$	Ring,
	8d cooler	2 <sup>3</sup> / <sub>8</sub>	0.281		-	Screw
0.120	-	_	-	-	$\begin{array}{c} 2, 2^{1}/_{4}, 2^{3}/_{8,} \\ 2^{1}/_{2}, 2^{3}/_{4,} 3, \\ 3^{1}/_{4,} 3^{1}/_{2}, 3^{3}/_{4,} \\ 4 \end{array}$	Smooth, Ring, Screw
	8d common	2 <sup>1</sup> / <sub>2</sub>	0.281	Smooth	1. 3.	
0.131	Metal Hardware <sup>2</sup>	$\begin{array}{c}1^{1}\!/_{4},1^{1}\!/_{2},\\2^{1}\!/_{4},2^{3}\!/_{8,}\\2^{1}\!/_{2}\end{array}$	0.281	Smooth, Ring	$\begin{array}{c} 2, 2^{1/_{4,}} 2^{3} /_{8,} \\ 2^{1/_{2,}} 2^{3/_{4,}} 3, \\ 3^{1/_{4,}} 3^{3} /_{8,} 3^{1/_{2,}} \\ 3^{3/_{4,}} 4 \end{array}$	Smooth, Ring, Screw
0.135	16d box	3 <sup>1</sup> / <sub>2</sub>	0.344	Smooth	2 <sup>3</sup> / <sub>8</sub> , 2 <sup>1</sup> / <sub>2</sub> , 3 <sup>1</sup> / <sub>2</sub>	Ring, Screw
	10d common	3	0.312	Crocoth		
	12d common	3 <sup>1</sup> / <sub>4</sub>	0.312	Smooth	2, $2^{1}/_{8}$ , $2^{1}/_{4}$ , $2^{3}/_{8}$ , $2^{1}/_{2}$ , 3,	Smooth,
0.148	Metal Hardware <sup>2</sup>	$1^{1}/_{4}, 1^{1}/_{2}, 2^{1}/_{2}, 3, 3^{1}/_{2},$	0.281	Smooth, Ring	2 <sup>3</sup> / <sub>8</sub> , 2 <sup>1</sup> / <sub>2</sub> , 3, 3 <sup>1</sup> / <sub>4</sub> , 3 <sup>1</sup> / <sub>2</sub> , 4	Ring, Screw
	16d common	3 <sup>1</sup> / <sub>2</sub>	0.344	Smooth		Smooth,
0.162	Metal Hardware <sup>2</sup>	$2^{1}/_{2}, 3, 3^{1}/_{2}$	0.281	Smooth, Ring	3, 3 <sup>1</sup> / <sub>4</sub> , 3 <sup>1</sup> / <sub>2</sub> , 4	Ring, Screw
0.180	-	-	_	-	5 <sup>3</sup> / <sub>8</sub>	Smooth
0.197	-	-	_	-	5 <sup>3</sup> / <sub>8</sub>	Smooth

#### TABLE 1—NAIL DIAMETERS ADDRESSED IN THIS REPORT<sup>1</sup>

For **SI:** 1 inch = 25.4 mm.

<sup>1</sup>See Appendix B for recognized nail products for each listee.

<sup>2</sup>Nails intended for use with metal hardware such as joist hangers. See Appendix B of this report for associated designations on product labels.

TABLE 2—APPLICABLE FASTENING SCHEDULES IN THE CODES FOR
ATTACHMENT OF SHEATHING TO FRAMING

CONSTRUCTION	CODE	TABLE NUMBER
	2018 and 2015 IBC	2304.10.1
Roof Sheathing Attachment	2012, 2009 and 2006 IBC	2304.9.1
	2018, 2015, 2012, 2009 and 2006 IRC	R602.3(1), R602.3(2)
	2018 and 2015 IBC	2304.10.1
Wall Sheathing Attachment	2012, 2009 and 2006 IBC	2304.9.1
	2018, 2015, 2012, 2009 and 2006 IRC	R602.3(1), R602.3(2), R602.3(3)
	2018 and 2015 IBC	2304.10.1
Floor Sheathing Attachment	2012, 2009 and 2006 IBC	2304.9.1
	2018, 2015, 2012, 2009 and 2006 IRC	R602.3(1), R602.3(2)

TABLE 3A—REFERENCE LATERAL DESIGN VALUES OF FACE NAILED SINGLE SHEAR CONNECTIONS OF "2-BY" MEMBERS TO OTHER MEMBERS OF SAME SPECIES  $^{1,2,3,4,5,6}$ 

NAIL D	IMENSIONS	REFERENCE LA	TERAL DESIGN VALU	IES FOR SPECIFIC GR	AVITIES OF: (lbf)
Length (inches)	Nail Shank Diameter (inches)	0.42 (e.g., Spruce- pine-fir)	0.43 (e.g., Hem-fir)	0.50 (e.g., Douglas Fir-larch)	0.55 (e.g., Southern Pine)
3 <sup>1</sup> / <sub>2</sub>	0.162	111	113	131	143
3 <sup>1</sup> / <sub>4</sub>	0.148	100	102	118	128
3	0.148	100	102	118	128
3 <sup>1</sup> / <sub>2</sub>	0.135	88	89	103	113
3 <sup>1</sup> / <sub>4</sub>	0.131	82	84	97	106
3	0.131	82	84	97	106
2 <sup>1</sup> / <sub>2</sub>	0.131	63	64	74	81
3 <sup>1</sup> / <sub>4</sub>	0.120	69	71	81	89
3	0.120	69	71	81	89
2 <sup>1</sup> / <sub>2</sub>	0.113	54	56	64	70
2 <sup>3</sup> / <sub>8</sub>	0.113	47	49	56	61
2 <sup>1</sup> / <sub>4</sub>	0.099	36	36	42	46

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45N, 1 psi = 6.89 kPa.

<sup>1</sup>Design values are based on a normal load duration.

<sup>2</sup>Table values must be multiplied by all applicable adjustment factors in the NDS.

<sup>3</sup>Table is based upon a 1<sup>1</sup>/<sub>2</sub>-inch actual thickness of both attached member and receiving ("main") member.

<sup>4</sup>Design values are for connections in which the nail shank is driven into the side grain with shank axis perpendicular to wood fibers. Tabulated values are based on a minimum fastener bending yield strength ( $F_{yb}$ ) of 100,000 psi for nail diameters of 0.135 inch or less, and a minimum fastener bending yield strength ( $F_{yb}$ ) of 90,000 psi for nail diameters of 0.148 and 0.162 inch.

<sup>5</sup>Calculations are based on a connection in which both members have the same specific gravity.

<sup>6</sup>Reference lateral design values apply to nails with either a smooth shank or a deformed shank.

#### TABLE 3B—REFERENCE LATERAL DESIGN VALUES OF FACE NAILED SINGLE SHEAR CONNECTIONS OF STEEL SIDE MEMBERS TO WOOD MEMBERS<sup>1,2,3,4</sup>

			REF	ERENC		AL DE	SIGN V	ALUES	FOR S	PECIFIC	GRAVI	ries⁵ o	F: (lbf)			
	0	.42 (e.g	., Sprud	ce-pine	-fir)	0.8	50 (e.g.,	Dougla	as Fir-la	arch)	0.55 (e.g., Southern Pine)					
STEEL SIDE		Nail D	)iamete	r (inch)	)	Nail Diameter (inch)					Nail Diameter (inch)					
MEMBER THICKNESS <sup>6</sup>	0.1	131	0.1	48	0.162	0.131		0.148		0.162	0.1	131	0.1	48	0.162	
(inch)		Nail L	ength (	inches	)		Nail L	ength (	inches	)		Nail L	ength (	inches	)	
	1 <sup>1</sup> / <sub>2</sub>	$2^{1}/_{4,}$ $2^{3}/_{8,}$ $2^{1}/_{2}$	<b>1</b> <sup>1</sup> / <sub>2</sub>	2 <sup>1</sup> / <sub>2,</sub> 3, 3 <sup>1</sup> / <sub>2</sub>	2 <sup>1</sup> / <sub>2</sub> , 3, 3 <sup>1</sup> / <sub>2</sub>	<b>1</b> <sup>1</sup> / <sub>2</sub>	$2^{1}/_{4,}$ $2^{3}/_{8,}$ $2^{1}/_{2}$	<b>1</b> <sup>1</sup> / <sub>2</sub>	2 <sup>1</sup> / <sub>2,</sub> 3, 3 <sup>1</sup> / <sub>2</sub>	2 <sup>1</sup> / <sub>2</sub> , 3, 3 <sup>1</sup> / <sub>2</sub>	<b>1</b> <sup>1</sup> / <sub>2</sub>	$2^{1}/_{4,}$ $2^{3}/_{8,}$ $2^{1}/_{2}$	<b>1</b> <sup>1</sup> / <sub>2</sub>	2 <sup>1</sup> / <sub>2,</sub> 3, 3 <sup>1</sup> / <sub>2</sub>	2 <sup>1</sup> / <sub>2</sub> , 3, 3 <sup>1</sup> / <sub>2</sub>	
				Á	STM A6	53, Gra	de 33 S	teel Sid	e Plate							
0.033 - 0.036	82	82	97	97	117	94	94	112	113	136	102	102	122	123	147	
0.044 - 0.048	83	83	97	98	117	95	95	112	114	136	102	102	122	124	148	
0.055 - 0.060	84	84	97	99	118	96	96	113	115	138	104	104	122	125	149	
0.068 - 0.075	86	86	98	102	121	98	98	114	118	140	106	106	123	127	151	
0.097 - 0.105	93	93	103	108	127	105	105	118	125	147	113	113	128	135	159	
0.127 - 0.134	102	102	109	118	137	115	115	126	135	157	124	124	135	146	170	
0.171 - 0.179	116	116	123	137	157	132	132	138	154	177	142	142	149	166	190	
0.228 - 0.240	111	116	119	140	168	127	132	137	160	192	138	144	148	174	209	
	•			Á	STM A6	53, Gra	de 40 S	teel Sid	e Plate							
0.033 - 0.036	83	83	97	98	117	95	95	113	114	137	103	103	123	124	149	
0.044 - 0.048	84	84	98	99	118	96	96	114	116	138	104	104	123	125	150	
0.055 - 0.060	86	86	99	101	120	98	98	115	117	141	106	106	124	127	151	
0.068 - 0.075	89	89	101	104	123	101	101	117	121	144	109	109	126	130	155	
0.097 - 0.105	97	97	107	113	132	110	110	123	130	155	118	118	133	140	164	
0.127 - 0.134	108	108	115	124	143	122	122	133	143	168	131	131	143	154	178	
0.171 - 0.179	116	116	127	141	167	133	133	145	161	193	145	145	157	175	203	
0.228 - 0.240	112	116	120	141	169	128	133	137	161	193	139	145	149	175	210	
					AST	M A36,	Steel S	ide Pla	te							
0.250	111	117	117	139	169	128	134	137	162	194	139	145	157	186	222	

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45N, 1 psi = 6.89 kPa.

<sup>1</sup>Design values are for normal load duration and must be multiplied by all applicable adjustment factors in the NDS.

<sup>2</sup>The tabulated values have been calculated in accordance with the Yield Mode Equations in Appendix A1.2. Dowel bearing strengths (F<sub>es</sub>) used to calculate design values are 61,850 psi for ASTM A653, Grade 33; 75,600 psi for ASTM A653 Grade 40; and 87,000 psi for ASTM A36 side member material.

<sup>3</sup>Lateral design values are based on  $F_{yb}$  = 100,000 psi for 0.131-inch diameter nails; and  $F_{yb}$  = 90,000 psi for 0.148 and 0.162-inch diameter nails.

<sup>4</sup>Wood member must be of sufficient thickness for the nail point to be fully embedded in the wood.

<sup>5</sup>Specific Gravity values must be the assigned specific gravity from Table A or 2018 or 2015 NDS Table 12.3.3A for the 2018 and 2015 IBC (2012 NDS Table 11.3.3A for the 2012 IBC, 2005 NDS Table 11.3.2A for the 2009 and 2006 IBC) or the equivalent specific gravity for engineered wood products, as shown in an ICC-ES evaluation report.

<sup>6</sup>These thicknesses are base metal thicknesses and are based on typical steel thicknesses recognized in various ICC-ES evaluation reports for metal hardware and on the thicknesses addressed in Table 12P of the 2018 and 2015 NDS.

	SMC	OOTH AN							(BRIGHT	OR	SMO	OTH AND			IANK ST. R IN INCH		STEEL N	AILS,	STAPLE GAGE AND DIAMETER <sup>6</sup> , in inches		
SPECIFIC GRAVITY <sup>4</sup>	0.092	0.099	0.113	0.120	0.131	0.135	0.148	0.162	0.180	0.197	0.092	0.099	0.113	0.120	0.131	0.135	0.148	0.162	16 gage	15 gage	14 gage
0.31	7	7	8	9	10	10	11	12	13	15	7	8	9	10	11	11	12	13	<b>0.063</b> 9	<b>0.072</b>	<b>0.080</b> 12
0.35	9	10	11	12	13	10	15	16	13	20	9	10	11	10	13	13	12	16	13	14	12
0.36	10	10	12	13	14	14	16	10	19	20	9	10	11	12	13	13	14	16	13	14	10
0.37	10	10	12	13	15	14	10	19	21	23	10	10	12	12	13	14	15	10	13	17	18
0.38	11	12	14	15	16	10	18	20	22	24	10	10	12	13	14	15	16	18	15	18	20
0.39	12	12	15	16	17	18	10	21	24	26	10	11	13	14	15	15	17	18	16	19	21
0.40	13	10	16	10	18	10	21	23	25	28	10	12	13	14	15	16	17	19	17	20	22
0.41	14	14	17	18	19	20	22	24	27	29	11	12	14	15	16	16	18	20	19	21	24
0.42	15	15	18	19	21	21	23	26	28	31	12	13	14	15	17	17	19	21	20	23	25
0.43	15	16	19	20	22	23	25	27	30	33	12	13	15	16	17	18	19	21	21	24	27
0.44	16	17	20	21	23	24	26	29	32	35	12	13	15	16	18	18	20	22	22	26	28
0.46	18	19	22	24	26	27	29	32	36	39	13	14	16	17	19	20	21	24	25	29	32
0.47	19	20	24	25	27	28	31	34	38	41	14	15	17	18	20	20	22	24	26	30	33
0.49	21	22	26	28	30	31	34	38	42	46	15	16	18	19	21	22	24	26	29	33	37
0.50	22	24	28	29	32	33	36	40	44	48	15	16	19	20	22	22	24	27	30	35	39
0.51	24	25	29	31	34	35	38	42	46	50	16	17	19	20	22	23	25	27	32	37	41
0.55	28	30	35	37	41	42	46	50	56	61	17	19	21	23	25	26	28	31	39	45	50
0.58	33	34	40	42	46	48	52	57	64	70	19	20	23	25	27	28	30	33	44	51	57
0.67	47	49	57	61	66	68	75	82	91	100	23	25	29	31	33	34	38	41	63	73	81
0.68	48	51	59	63	69	71	78	85	95	104	24	26	29	31	34	35	39	42	66	76	84
0.71	54	57	66	70	77	79	87	95	106	115	26	28	31	33	36	38	41	45	73	84	94
0.73	58	61	71	75	82	85	93	102	113	124	27	29	33	35	38	39	43	47	79	90	101

#### TABLE 4—NAIL AND STAPLE REFERENCE WITHDRAWAL DESIGN VALUES<sup>1,2,3</sup> (pounds-force per inch of penetration)

For SI: 1 inch = 25.4 mm, 1 pound-force per inch = 0.175 N/mm.

<sup>1</sup>Design values are based on a normal (10 year) duration of load.

<sup>2</sup>Table values must be multiplied by all applicable adjustment factors in the NDS.

<sup>3</sup>Withdrawal strengths are for fasteners driven perpendicular to the grain.

<sup>4</sup>Specific Gravity values must be the assigned specific gravity from Table A or 2018 or 2015 NDS Table 12.3.3A for the 2015 IBC (2012 NDS Table 11.3.3A for the 2012 IBC, 2005 NDS Table 11.3.2A for the 2009 and 2006 IBC) or the equivalent specific gravity for engineered wood products, as shown in an ICC-ES evaluation report.

<sup>5</sup>Applies to deformed nails recognized in this report.

<sup>6</sup>Values account for both staple legs.

# TABLE 5—ALLOWABLE SHEAR FOR WIND OR SEISMIC LOADING FOR WOOD STRUCTURAL PANEL HORIZONTAL DIAPHRAGMS WITH FRAMING OF DOUGLAS FIR-LARCH OR SOUTHERN PINE AND STRUCTURAL I SHEATHING (plf)<sup>1,2,3,4,5,6,7,8,9</sup>

NOMINAL NAIL					В		IAPHRAGM	s			UNBLOCKED DIAPHRAGMS				
DIAMETER (inch) or STAPLE GAGE	MINIMUM REQUIRED FASTENER	MINIMUM WIDTH OF			CING (inch) L EDGES P/		FASTENERS SPACED 6" MAX. AT SUPPORTED EDGES								
Nails must be smooth		FRAMING	6	i	4	ļ	<b>2</b> <sup>1</sup>	l <sub>2</sub>	2	-	Case	<b>1</b>	All o		
or deformed, and must	LENGTH (inches)	MEMBER (inches)		Nai	I spacing at	other panel	el edges (Ca	ses 1, 2, 3	& 4)		unblocked	•	•	configurations (Cases 2, 3, 4,	
be carbon steel (bright or galvanized).	(	(	6	;	6	6	4	ļ	3	5	continuous joints parallel to load)		5 8		
			Seismic	Wind	Seismic	Wind	Seismic	Wind	Seismic	Wind	Seismic	Wind	Seismic	Wind	
				<sup>3</sup> / <sub>8</sub> -ir	nch Nomina	I Panel Th	nickness								
0.131	1 <sup>3</sup> / <sub>4</sub>	2 3	270 300	375 420	360 400	505 560	530 600	740 840	600 675	840 945	240 265	335 370	180 200	255 280	
0.120	1 <sup>3</sup> / <sub>4</sub>	2 3	230 255	320 360	305 340	435 480	455 510	635 720	515 580	720 810	200 225	290 320	150 170	220 240	
0.113	1 <sup>3</sup> / <sub>4</sub>	2 3	205 230	290 325	275 305	390 430	410 460	570 645	465 520	645 725	180 205	260 285	135 155	200 215	
14, 15, 16 Gage	1 <sup>1</sup> / <sub>2</sub> Leg Length	2 3	175 200	245 280	235 265	330 370	350 395	490 550	400 450	560 630	155 175	215 245	115 130	160 180	
	•			<sup>15</sup> / <sub>32</sub> -i	nch Nomin	al Panel T	hickness								
0.148 smooth	2	2 3	320 360	445 505	425 480	595 670	640 720	895 1005	730 820	1025 1150	285 320	400 445	215 240	300 335	
0.135	2	2 3	285 320	395 450	380 430	530 595	570 640	795 895	650 730	910 1020	255 285	355 395	195 215	270 300	
0.131	2	2 3	270 305	375 425	360 405	505 565	540 605	755 845	610 685	865 970	240 270	340 375	180 200	255 285	
0.120	2	2 3	230 260	325 370	310 350	435 490	465 520	650 730	525 590	745 835	205 230	290 325	155 175	220 245	
0.113	2	2 3	210 235	295 335	280 315	395 440	420 470	590 660	475 535	675 755	185 210	265 295	140 155	200 220	
14, 15, 16 Gage	1 <sup>1</sup> / <sub>2</sub> Leg Length	2 3	175 200	245 280	235 265	330 370	350 395	490 550	400 450	560 630	155 175	215 245	120 130	160 180	

See page 11 for footnote explanations and case diagrams.

# TABLE 6—ALLOWABLE SHEAR FOR WIND OR SEISMIC LOADING FOR WOOD STRUCTURAL PANEL HORIZONTAL DIAPHRAGMS WITH FRAMING OF DOUGLAS FIR-LARCH OR SOUTHERN PINE AND RATED SHEATHING (pif)<sup>1,2,3,4,5,6,7,8,9</sup>

NOMINAL NAIL					E	UNBLOCKED DIAPHRAGMS								
DIAMETER (inch) or	MINIMUM	MINIMUM							ES), AT CON L EDGES (CA		FASTI		ACED 6" MA ED EDGES	X. AT
STAPLE GAGE	REQUIRED FASTENER	WIDTH OF FRAMING		6	4	ļ	2 <sup>1</sup>	l <sub>2</sub>	2	2	Case		All other	
Nails must be smooth or deformed and must be carbon	LENGTH	MEMBER		Ν	lail spacing a	t other pane	el edges (Cas	es 1, 2, 3 &	4)		unblocked edges or continuous joints		configurations (Cases 2, 3, 4,	
steel (bright or galvanized).	(inches)	(inches)	6		6		4 3		3	parallel		5 8		
			Seismic	Wind	Seismic	Wind	Seismic	Wind	Seismic	Wind	Seismic	Wind	Seismic	Wind
	-	_		<sup>3</sup> / <sub>8</sub> -	inch Nominal	Panel Thicl	iness							
0.131	1 <sup>3</sup> / <sub>4</sub>	2 3	240 270	335 375	320 360	445 505	480 540	670 755	545 610	760 855	215 240	300 335	160 180	225 250
0.120	1 <sup>3</sup> / <sub>4</sub>	2 3	205 230	285 315	270 305	375 425	405 455	565 640	460 515	640 720	180 205	255 285	135 150	190 210
0.113	1 <sup>3</sup> / <sub>4</sub>	2 3	180 205	255 285	240 270	335 380	360 405	505 570	410 460	575 645	160 180	225 255	120 135	170 190
14, 15,16 Gage	1 <sup>1</sup> / <sub>2</sub> Leg Length	2 3	160 180	225 250	210 235	295 330	315 355	440 495	360 400	505 560	140 160	195 225	105 120	145 170
		•		<sup>7</sup> / <sub>16</sub> -	inch Nomina	I Panel Thic	kness							
0.131	2	2 3	255 285	360 400	340 380	475 530	505 570	705 800	575 645	805 900	230 255	320 355	170 190	235 265
0.120	2	2 3	215 240	305 340	290 325	405 450	430 485	600 680	490 550	685 765	190 215	270 300	145 160	200 225
0.113	2	2 3	195 215	275 305	260 290	360 405	385 435	540 610	440 490	615 685	175 195	245 270	130 145	180 200
14, 15, 16 Gage	1 <sup>1</sup> / <sub>2</sub> Leg Length	2 3	165 190	230 265	225 250	315 350	335 375	470 525	380 425	530 595	150 165	210 230	110 125	155 175
		•		<sup>15</sup> / <sub>32</sub>	-inch Nomina	I Panel Thic	kness							
0.148	2	2 3	290 325	405 455	385 430	540 605	575 650	805 910	655 735	920 1030	255 290	360 405	190 215	265 300
0.135	2	2 3	255 285	355 400	340 380	475 530	505 575	710 800	580 650	810 910	225 255	315 355	170 190	235 265
0.131	2	2 3	270 300	380 420	360 400	505 560	530 600	740 840	600 675	840 945	240 265	335 370	180 200	255 280
0.120	2	2 3	230 255	325 360	305 340	430 480	450510	630 715	510 575	715 805	205 225	285 315	155 170	220 240
0.113	2	2 3	205 230	290 320	275 305	385 430	405 460	570 645	460 520	645 725	185 205	255 285	140 155	195 215
14, 15, 16 Gage	1 <sup>1</sup> / <sub>2</sub> Leg Length	2 3	160 180	225 250	210 235	295 330	315 355	440 495	360 405	505 565	140 160	195 225	105 120	145 170
		•		<sup>19</sup> / <sub>32</sub> -	inch Nominal	Panel Thick	ness <sup>10</sup>							
0.148	2 <sup>1</sup> / <sub>4</sub>	2 3	320 360	445 505	425 480	595 675	640 720	895 1010	730 820	1025 1150	285 320	400 445	215 240	300 335
0.135	2 <sup>1</sup> / <sub>4</sub>	2 3	285 320	395 450	375 425	525 595	565 640	795 895	645 725	905 1020	255 285	355 395	190 215	265 295
0.131	2 <sup>1</sup> / <sub>4</sub>	2 3	270 305	375 425	360 405	500 565	540 605	755 850	615 690	860 965	240 270	335 375	180 200	255 285
0.120	2 <sup>1</sup> / <sub>4</sub>	2 3	235 260	325 365	310 350	435 490	465 525	650 735	530 595	745 835	205 235	290 325	155 175	220 245
0.113	2 <sup>1</sup> / <sub>4</sub>	2 3	210 240	295 335	280 315	395 445	420 475	590 665	480 540	675 760	190 210	265 295	140 160	200 220
14, 15, 16 Gage	1 <sup>1</sup> / <sub>2</sub> Leg Length	2 3	175 200	245 280	235 265	330 370	350 395	490 555	400 450	560 630	155 175	215 245	115 130	160 180

See page 11 for footnote explanations and case diagrams.

#### FOOTNOTE EXPLANATIONS FOR HORIZONTAL DIAPHRAGM TABLES 5 AND 6

<sup>1</sup>For **SI:** 1 inch = 25.4 mm, 1 plf = 14.6 N/m.

<sup>2</sup>Diaphragm construction using nails must be in accordance with Section 4.2.6 and 4.2.7 of the 2015 and 2008 AWC Special Design Provisions for Wind and Seismic (SPDWS), and diaphragm construction using staples must be in accordance with 2018 and 2015 IBC Tables 2306.2(1) and 2306.2(2) (similar for earlier codes), as applicable.

<sup>3</sup>Tabulated values are for short-time loading due to wind or seismic. The tabulated seismic values must be reduced by 37 percent and 44 percent for normal and permanent load duration, respectively.

<sup>4</sup>The tabulated values are for fasteners installed in Douglas Fir-larch or Southern Pine framing. For framing of other species: (1) Find the assigned specific gravity for the applicable species of lumber (see Section A1.3). (2) For staples find the shear value from Table 5 (regardless of actual sheathing grade) and multiply the value by 0.82 for species with specific gravity of 0.42 or greater, or by 0.65 for all other species. (3) For nails find the shear value from the applicable table and multiply value by the Specific Gravity Adjustment Factor = [1-(0.5 - G)], where G = Specific Gravity of the framing lumber. This adjustment factor must not be greater than 1.

<sup>5</sup>Diaphragm deflection must be determined in accordance with Section A3.0.

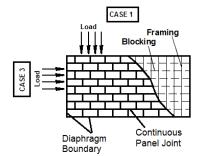
<sup>6</sup>Structural I panels must comply with DOC PS1 or PS2. Rated Sheathing includes Sheathing and Single-Floor grades and must comply with DOC PS1 or PS2.

<sup>7</sup>Nails must be bright or galvanized carbon steel, flat head nails denoted in Appendix B as meeting the head area ratio requirements for lateral force resisting assemblies. A deformed shank nail must have either a helical (screw) shank or an annular (ring) shank. Diaphragm values for stainless steel nails are outside the scope of this report.

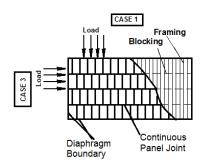
<sup>8</sup>Staples must have a <sup>7</sup>/<sub>16</sub>-inch minimum crown width and must be installed with their crowns parallel to the long dimension of the framing members and must be driven flush with the surface of the sheathing.

<sup>9</sup>Space fasteners maximum 12" o.c. along intermediate framing members (6 in. o.c. when supports are spaced 48 inches o.c.).

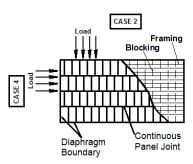
<sup>10</sup>Tabulated values apply to wood structural panels up to  $1^{1}/_{8}$ " in thickness, provided the nail penetration is at least  $1^{1}/_{2}$  inches and the staple penetration is at least 1 inch.



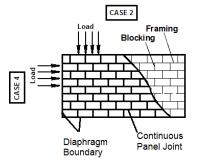
Continuous Panel Joints Perpendicular to Framing Long Panel Direction Perpendicular to Support



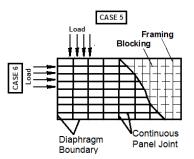
Continuous Panel Joints Perpendicular to Framing Long Panel Direction Parallel to Supports



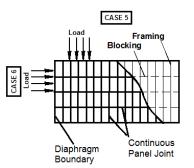
Continuous Panel Joints Parallel to Framing Long Panel Direction Perpendicular to Supports

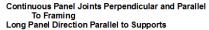


Continuous Panel Joints Parallel to Framing Long Panel Direction Parallel to Supports



Continuous Panel Joints Perpendicular and Parallel To Framing Long Panel Direction Perpendicular to Supports





CASE DIAGRAMS FOR HORIZONTAL DIAPHRAGM TABLES 5 AND 6

# TABLE 7—ALLOWABLE SHEAR FOR WIND OR SEISMIC LOADING FOR WOOD STRUCTURAL PANEL SHEAR WALLS WITH FRAMING OF DOUGLAS FIR-LARCH OR SOUTHERN PINE AND STRUCTURAL I SHEATHING (plf)<sup>1,2,3,4,5,6,7,8,9,10,11</sup>

NOMINAL NAIL DIAMETER (inch) or	FAST	NOMINAL ENER I (inches)		SEIS	SMIC			WIND						
STAPLE GAGE Nails must be smooth and must be carbon steel (bright or galvanized)	Panels Applied Directly	Panels Applied Over <sup>1</sup> / <sub>2</sub> inch or <sup>5</sup> / <sub>8</sub> inch		Spacing at	Panel Edge	s (inches) 2	Fastener 6	Spacing at	Panel Edge 3	s (inches)				
	to Framing	Gypsum Sheathing	-		-					_				
			<sup>3</sup> / <sub>8</sub> -inch No	minal Pane	I Thicknes	s			r	1				
0.148	2	—	230	360	460	610	320	505	645	855				
0.140		2 <sup>1</sup> / <sub>2</sub>	280	430	550	730	390	600	770	1020				
0.135	2	—	230	360	460	610	320	505	645	855				
0.155	—	2 <sup>1</sup> / <sub>2</sub>	250	380	485	645	345	530	680	900				
0.131	1 <sup>3</sup> / <sub>4</sub>	—	230	360	460	610	320	505	645	855				
0.131	—	2 <sup>1</sup> / <sub>2</sub>	235	360	460	610	330	505	645	855				
0.120	1 <sup>3</sup> / <sub>4</sub>	—	200	310	395	520	275	435	550	730				
0.120		2 <sup>1</sup> / <sub>2</sub>	200	310	395	520	280	430	550	725				
0.440	1 <sup>3</sup> / <sub>4</sub>	_	180	280	355	470	245	390	495	655				
0.113	—	2 <sup>1</sup> / <sub>2</sub>	180	275	355	470	250	385	495	655				
14, 15, 16 Gage	1 <sup>1</sup> / <sub>2</sub>	_	155	235	315	400	215	330	440	560				
14, 15, 16 Gage	_	2	155	235	310	400	215	330	435	560				
, , <b>, ,</b>		7	/ / <sub>16</sub> -inch No	minal Pane	el Thicknes	s	I		l					
	2	_	260	395	505	670	355	550	705	935				
0.148		2 <sup>1</sup> / <sub>2</sub>	280	430	550	730	390	600	770	1020				
	2		260	395	505	670	355	550	705	935				
0.135		2 <sup>1</sup> / <sub>2</sub>	250	385	490	650	345	535	685	905				
	2	2	260	395	505	670	355	550	705	935				
0.131		2 <sup>1</sup> / <sub>2</sub>	235	365	465	615	330	505	650	860				
	2	_ 12	225	340	435	580	305	475	610	805				
0.120		2 <sup>1</sup> / <sub>2</sub>	205	310	400	530	285	435	555	735				
	2	<i>L</i> 12	200	310	395	520	280	430	550	730				
0.113		2 <sup>1</sup> / <sub>2</sub>	170	260	330	440	235	360	460	610				
14, 15, 16 Gage	1 <sup>1</sup> / <sub>2</sub>	<i>L</i> 12	170	260	345	440	240	365	485	615				
14, 15, 16 Gage	172	2	155	235	343	400	240	330	435	560				
14, 15, 10 Gage	_				el Thicknes		215	550	400	500				
	2		340	510	665	870	475	715	930	1215				
0.148	2	 2 <sup>1</sup> / <sub>2</sub>	280	430	550	730	390	600	770	1020				
	-	212												
0.135	2		305	455	590	775	425	635	825	1080				
	-	2 <sup>1</sup> / <sub>2</sub>	250	385	490	650	350	535	685	905				
0.131	2		280	430	550	730	390	600	770	1020				
	-	2 <sup>1</sup> / <sub>2</sub>	240	365	465	615	330	505	650	860				
0.120	2	-1	245	375	475	630	340	520	665	880				
	—	2 <sup>1</sup> / <sub>2</sub>	205	315	400	530	285	435	560	740				
0.113	2	_	220	340	430	570	305	470	605	800				
	—	2 <sup>1</sup> / <sub>2</sub>	185	285	365	480	260	395	510	670				
14, 15, 16 Gage	1 <sup>1</sup> / <sub>2</sub>	—	185	280	375	475	260	390	525	665				
14, 15, 16 Gage	—	2	155	235	300	400	215	330	420	560				

See page 14 for footnote explanations.

# TABLE 8—ALLOWABLE SHEAR FOR WIND OR SEISMIC LOADING FOR WOOD STRUCTURAL PANEL SHEAR WALLSWITH FRAMING OF DOUGLAS FIR-LARCH OR SOUTHERN PINE AND RATED SHEATHING (pif)

NOMINAL NAIL DIAMETER (inch) or	FAST	NOMINAL ENER I (inches)		SEI	SMIC		WIND					
STAPLE GAGE		Panels	Fastener	Spacing at	Panel Edge	s (inches)	Fastener	Spacing at	Panel Edge	s (inches)		
Nails must be smooth and must be carbon steel (bright or galvanized)	Panels Applied Directly to Framing	Applied Over <sup>1</sup> / <sub>2</sub> inch or <sup>5</sup> / <sub>8</sub> inch Gypsum Sheathing	6	4	3	2	6	4	3	2		
		;	<sup>3</sup> / <sub>8</sub> -inch No	minal Pane	I Thicknes	S	•	•				
0.4.40	2	_	220	320	410	530	305	445	575	740		
0.148	_	2 <sup>1</sup> / <sub>2</sub>	260	380	490	640	365	530	685	895		
0.405	2	_	220	320	410	530	305	445	575	740		
0.135	_	2 <sup>1</sup> / <sub>2</sub>	230	335	430	560	320	465	600	785		
0.404	1 <sup>3</sup> / <sub>4</sub>	—	220	320	410	530	305	445	575	740		
0.131	_	2 <sup>1</sup> / <sub>4</sub>	200	300	390	510	280	420	545	715		
	1 <sup>3</sup> / <sub>4</sub>	_	185	270	345	450	260	375	485	625		
0.120	_	2 <sup>1</sup> / <sub>2</sub>	170	255	330	430	235	355	460	605		
0.440	1 <sup>3</sup> / <sub>4</sub>	—	200	300	390	510	280	420	545	715		
0.113	_	2 <sup>1</sup> / <sub>4</sub>	150	225	295	385	210	315	410	540		
14, 15, 16 Gage	1 <sup>1</sup> / <sub>2</sub>	_	140	210	280	360	195	295	390	505		
14, 15, 16 Gage	_	2	140	210	280	360	195	295	390	505		
, -, - <del></del> g-					el Thicknes							
	2 <sup>1</sup> / <sub>2</sub>	_	240	350	450	585	335	490	630	820		
0.148	_ /2	2 <sup>1</sup> / <sub>2</sub>	260	380	490	640	365	530	685	895		
	2		240	350	450	585	335	490	630	820		
0.135		2 <sup>1</sup> / <sub>2</sub>	230	335	435	565	320	465	605	790		
	2	<b>Z</b> 12	240	350	450	585	335	490	630	820		
0.131		2 <sup>1</sup> / <sub>2</sub>	240	315	410	535	305	440	570	745		
	2	2 12	205	300	385	495	285	415	535	695		
0.120		2 <sup>1</sup> / <sub>2</sub>	185	270	345	455	260	375	485	635		
	2	2 12	185	265	345	445	255	375	480	625		
0.113	2	 2 <sup>1</sup> / <sub>2</sub>	165	203	343	445	230	335	435	570		
14, 15, 16 Gage			155	240	310	395	230	320	435	555		
14, 15, 16 Gage	1 /2	2	135	230	280	395	195	295	435 390	505		
14, 15, 10 Gaye					el Thicknes		195	295	390	505		
	2					770	425	C 4 E	940	1075		
0.148	2		310	460	600		435	645 530	840			
	-	2 <sup>1</sup> / <sub>2</sub>	260	380	490	640	365	530	685	895		
0.135	2		275	405	530	680	385	570	740	950		
	_	2 <sup>1</sup> / <sub>2</sub>	230	335	430	565	320	465	605	790		
0.131	2		260	380	490	640	365	530	685	895		
	-	2 <sup>1</sup> / <sub>2</sub>	215	315	410	535	305	440	570	745		
0.120	2	-	220	325	420	545	310	450	585	765		
	_	2 <sup>1</sup> / <sub>2</sub>	185	270	350	455	260	375	490	635		
0.113	2	-	200	290	375	490	280	405	525	685		
		2 <sup>1</sup> / <sub>2</sub>	165	245	315	410	235	340	440	575		
14, 15, 16 Gage	1 <sup>1</sup> / <sub>2</sub>	_	170	255	335	430	240	355	470	600		
14, 15, 16 Gage	—	2	140	210	280	360	195	295	390	505		
	o <sup>1</sup> .	13		T	el Thicknes			<b>_</b> · -	0.00	46.1-		
0.148	2 <sup>1</sup> / <sub>4</sub>	—	340	510	665	870	475	715	930	1215		
0.135	2 <sup>1</sup> / <sub>4</sub>	—	300	450	590	770	420	635	825	1075		
0.131	2 <sup>1</sup> / <sub>4</sub>	—	285	430	560	735	400	600	785	1025		
0.120	2 <sup>1</sup> / <sub>4</sub>	—	245	370	485	635	345	520	675	885		
0.113	2 <sup>1</sup> / <sub>4</sub>	—	225	335	440	575	315	470	615	800		
14, 15, 16 Gage	1 <sup>3</sup> / <sub>4</sub>		185	280	375	475	260	390	525	665		

#### FOOTNOTE EXPLANATIONS FOR SHEAR WALL TABLES 7 AND 8

#### <sup>1</sup>For **SI:** 1 inch = 25.4 mm, 1 plf = 14.6 N/m.

<sup>2</sup>Shear wall construction using nails must be in accordance with Section 4.3.6 and 4.3.7 of the 2015 and 2008 AWC/AF&PA Special Design Provisions for Wind and Seismic (SPDWS), and shear wall construction using staples must be in accordance with 2018 and 2015 IBC Table 2306.3(1) (similar for earlier codes), as applicable.

<sup>3</sup>Tabulated values are for short-time loading due to wind or seismic. The tabulated seismic values must be reduced by 37 percent and 44 percent for normal and permanent load duration, respectively.

<sup>4</sup>The tabulated values are for fasteners installed in Douglas Fir-larch or Southern Pine. For framing of other species: (1) Find the assigned specific gravity for species of lumber (see Section A1.3) (2) For staples find shear value from Table 7 (regardless of actual sheathing grade) and multiply value by 0.82 for species with specific gravity of 0.42 or greater, or 0.65 for all other species. (3) For nails find shear value from the applicable table and multiply by the following Specific Gravity Adjustment Factor = [1 - (0.5 - G)], where *G* = Assigned Specific Gravity of the framing lumber. This adjustment factor must not be greater than 1.

<sup>5</sup>Shear wall deflection must be determined in accordance with Section A3.0.

<sup>6</sup>Structural I and Rated Sheathing panels must comply with DOC PS1 or PS2. Install panels either horizontally or vertically. All panel edges must be backed by framing members.

<sup>7</sup>In structures assigned to Seismic Design category D, E, or F, where the allowable shear design value exceeds 350 plf, all framing members receiving edge nailing from abutting panels must not be less than a single 3-inch nominal member. Panel joint and sill plate nailing must be staggered in all cases. See Section 4.3.6.4 of SDPWS, or 2006 IBC Section 2305.3.11 for sill plate size and anchorage requirements, as applicable.

<sup>8</sup>Space fasteners maximum 6 inches on center along intermediate framing members - Exception: When panel thickness is greater than  $7/_{16}$ -inch or studs are spaced less than 24 inches on center, space fasteners maximum 12 inches on center.

<sup>9</sup>Nails must be bright or galvanized carbon steel, flat head nails denoted in Appendix B as meeting the head area ratio requirements for lateral force resisting assemblies. A deformed shank nail must have either a helical (screw) shank or an annular (ring) shank. Shear wall values for stainless steel nails are outside the scope of this report.

<sup>10</sup>Staples must have a <sup>7</sup>/<sub>16</sub>-inch minimum crown width and must be installed with their crown parallel to the long dimension of the framing members, and must be driven flush with the surface of the sheathing.

<sup>11</sup>The values for  ${}^{3}_{/8}$ -inch and  ${}^{7}_{/16}$ -inch panels applied directly to framing using nails may be increased to values shown for  ${}^{15}_{/32}$ -inch-thick panels of the same panel grade, provided studs are spaced a maximum of 16 inches on center or panels are applied with long dimension across studs.

# TABLE 9—ALLOWABLE SHEAR FOR WIND OR SEISMIC LOADING FOR SHEAR WALLS WITH FIBERBOARD SHEATHING, GYPSUM LATH, GYPSUM SHEATHING, GYPSUM WALLBOARD, LATH AND PLASTER OR PLYWOOD SIDING OVER WOOD FRAMING (plf)<sup>1,3,4,5</sup>

SHEATHING	THICKNESS	WALL	REQU SPAC (inches or	ING	SHEAR (pl		FASTENER	COMMENTS							
MATERIAL	OF MATERIAL	CONSTRUCTION	Panel Edges	Field	Seismic	Wind	SPECIFICATIONS								
			4		150	210	41/11/11 40 45 0 44								
			3		200	280	1 <sup>1</sup> / <sub>4</sub> " long, 16, 15 & 14 gage staple								
	<sup>1</sup> / <sub>2</sub> "	Blocked	2	6	225	315	Stapic								
	12	BIUCKEU	4	0	220	310	11/ " lang 1" arours								
			3		290	405	1 <sup>1</sup> / <sub>4</sub> " long, 1" crown, 16, 15 & 14 gage staple	Reference 2018, 2015 and							
Fiberboard			2		325	455		2012 IBC Table 2306.3(2)							
Sheathing			4		150	210	$1^{1}/_{2}$ " long, 16, 15 & 14 gage	[2009 IBC Table 2306.6]							
			3		200	280	staple	for applicable notes							
	<sup>25</sup> / <sub>32</sub> "	Blocked	2	6	225	315		-							
	• 52	Dioditida	4		220	310	1 <sup>1</sup> / <sub>2</sub> " long, 1" crown,								
			3		290	405	16, 15 & 14 gage staple								
	2		2		325	455									
Gypsum Lath	<sup>3</sup> / <sub>8</sub> " + <sup>1</sup> / <sub>2</sub> " Plaster	Unblocked	5		10		1 <sup>1</sup> / <sub>8</sub> " long, <sup>3</sup> / <sub>4</sub> " crown, 16, 15 & 14 gage staple								
Gypsum	<sup>1</sup> / <sub>2</sub> " x 2' x 8'	Unblocked	4		7		$1^{3}/_{4}$ " long, 16, 15 & 14 gage								
Sheathing	<sup>1</sup> / <sub>2</sub> " x 4'	Blocked			17		staple								
	-	Unblocked	7		10										
			7		75		-								
		<sup>1</sup> / <sub>2</sub> "			10										
	<sup>1</sup> / <sub>2</sub> "				11		$1^{1}/_{2}$ " long, 16, 15 & 14 gage								
					12		staple								
		Blocked	7		12		-	Reference 2018, 2015 and							
Gypsum			4		15 11			2012 IBC Table 2306.3(3)							
Wallboard	-			Unblocked	4					[2009 IBC Table 2306.7]					
				4 7		145 <sup>2</sup> 145		1 <sup>5</sup> / <sub>8</sub> " long, 16, 15 & 14 gage staple	for applicable notes						
	5	Blocked	4		145		Stapic								
	°/8"	<sup>5</sup> / <sub>8</sub> "		Base Ply - 9		5	1 <sup>5</sup> / <sub>8</sub> " long, 16, 15 & 14 gage staple	-							
		Blocked two-ply	Face Ply - 7		250		2 <sup>1</sup> / <sub>4</sub> " long, 15 & 14 gage staple	-							
Expanded metal or woven wire lath and Portland cement plaster	7/ <sub>8</sub> "	Unblocked	6" On Ce Each Fi Mem	aming	18	I	$^{7}$ / <sub>8</sub> " long, $^{3}$ / <sub>4</sub> " crown, 16, 15 & 14 gage staple	1							
			6	l	160	225									
						-		-	-	4	6	240	335	$2^{1}/_{2} \times 0.113$ smooth nail	Reference SDPWS Table
						435	(carbon steel)	4.3A for applicable notes							
			3		310		(carbon steel)								
		Panels Applied	2		410	575									
		Panels Applied Directly To Framing	2 6		410 140	575 195		Reference 2018, 2015 and							
Plywood Panel			2 6 4	6	410 140 210	575 195 295	1 <sup>1</sup> / <sub>2</sub> " long, 16, 15 & 14 gage	Reference 2018, 2015 and 2012 IBC Table 2306.3(1)							
Siding Shear			2 6 4 3	6	410 140 210 280	575 195 295 390		Reference 2018, 2015 and 2012 IBC Table 2306.3(1) [2009 IBC Table 2306.3]							
Siding Shear Walls with Framing of	<sup>3</sup> /8"		2 6 4 3 2	6	410 140 210 280 360	575 195 295 390 505	1 <sup>1</sup> / <sub>2</sub> " long, 16, 15 & 14 gage	Reference 2018, 2015 and 2012 IBC Table 2306.3(1)							
Siding Shear Walls with Framing of Douglas Fir-	<sup>3</sup> / <sub>6</sub> "		2 6 4 3 2 6	6	410 140 210 280 360 160	575 195 295 390 505 225	1 <sup>1</sup> / <sub>2</sub> " long, 16, 15 & 14 gage staple	Reference 2018, 2015 and 2012 IBC Table 2306.3(1) [2009 IBC Table 2306.3] for applicable notes							
Siding Shear Walls with Framing of Douglas Fir- Larch or	3/8"		2 6 4 3 2 6 4	6	410 140 210 280 360 160 240	575 195 295 390 505 225 335	1 <sup>1</sup> / <sub>2</sub> " long, 16, 15 & 14 gage staple 3 x 0.131 smooth nail	Reference 2018, 2015 and 2012 IBC Table 2306.3(1) [2009 IBC Table 2306.3] for applicable notes Reference SDPWS Table							
Siding Shear Walls with Framing of Douglas Fir-	3/8"	Directly To Framing	2 6 4 3 2 6 4 3		410 140 210 280 360 160 240 310	575 195 295 390 505 225 335 435	1 <sup>1</sup> / <sub>2</sub> " long, 16, 15 & 14 gage staple	Reference 2018, 2015 and 2012 IBC Table 2306.3(1) [2009 IBC Table 2306.3] for applicable notes							
Siding Shear Walls with Framing of Douglas Fir- Larch or	3/ <sub>8</sub> "	Directly To Framing Panels Applied Over <sup>1</sup> / <sub>2</sub> " or <sup>5</sup> / <sub>8</sub> " Gypsum	2 6 4 3 2 6 4 3 2		410 140 210 280 360 160 240 310 410	575 195 295 390 505 225 335 435 575	1 <sup>1</sup> / <sub>2</sub> " long, 16, 15 & 14 gage staple 3 x 0.131 smooth nail	Reference 2018, 2015 and 2012 IBC Table 2306.3(1) [2009 IBC Table 2306.3] for applicable notes Reference SDPWS Table 4.3B for applicable notes							
Siding Shear Walls with Framing of Douglas Fir- Larch or	3/ <sub>8</sub> "	Directly To Framing	2 6 4 3 2 6 4 3 2 6 6		410 140 210 280 360 160 240 310 410 140	575 195 295 390 505 225 335 435 575 195	1 <sup>1</sup> / <sub>2</sub> " long, 16, 15 & 14 gage staple 3 x 0.131 smooth nail (carbon steel)	Reference 2018, 2015 and 2012 IBC Table 2306.3(1) [2009 IBC Table 2306.3] for applicable notes Reference SDPWS Table 4.3B for applicable notes Reference 2018, 2015 and							
Siding Shear Walls with Framing of Douglas Fir- Larch or	<sup>3</sup> /8"	Directly To Framing Panels Applied Over <sup>1</sup> / <sub>2</sub> " or <sup>5</sup> / <sub>8</sub> " Gypsum	2 6 4 3 2 6 4 3 2		410 140 210 280 360 160 240 310 410	575 195 295 390 505 225 335 435 575	1 <sup>1</sup> / <sub>2</sub> " long, 16, 15 & 14 gage staple 3 x 0.131 smooth nail	Reference 2018, 2015 and 2012 IBC Table 2306.3(1) [2009 IBC Table 2306.3] for applicable notes Reference SDPWS Table							

For SI: 1 inch = 25.4 mm; 1 foot = 305 mm; 1 plf = 14.6 N/m.

<sup>1</sup>Shear values are based on maximum framing spacing of 16 inches on center, unless otherwise noted.

<sup>2</sup>Shear values are based on maximum framing spacing of 24 inches on center.

<sup>3</sup>Staples must have a minimum crown width of  $\frac{7}{16}$  inch, measured outside the legs, unless otherwise noted.

<sup>4</sup>Nails must be bright or galvanized carbon steel, flat head nails denoted in Appendix B as meeting the head area ratio requirements for lateral force resisting assemblies. Shear wall values for stainless steel nails are outside the scope of this report.

<sup>5</sup>In addition to requirements presented above for fastening of shear walls all other requirements of the applicable model code (such as, but not limited to, conditions of use and modification of design values for certain Seismic Design Categories) pertaining to shear wall design and construction must be met.

	MINIMUM	FASTENING REQUIRE	MENTS PRESCRIBED IN 1	THE CODE	ALTERNA	TIVE FASTENING REQUI	REMENTS
CONNECTION	2012 IBC	2012 IRC	2018 and 2015 IBC	2018 and 2015 IRC	Nails may be carbo	n steel or stainless steel, o	except where noted
DESCRIPTION	Table 2304.9.1	Table R602.3(1)	Table 2304.10.1	Table R602.3(1)		otherwise.	
	" Nail Size	Noil Size	# Nail Size	# Nail Size	# Nail Size	# Nail Size	" Nail Size
	# (Type, inch)	# (Type, inch)	# (Type, inch)	# (Type, inch)	# (Type, inch)	# (Type, inch)	# (Type, inch)
	Connection 9	Connection 12	Connection 8	Connection 8			
Stud-to-stud	@ 24" o.c.	@ 24" o.c.	@ 24" o.c.	@ 24" o.c.	@ 24" o.c.	@ 16" o.c.	@ 8" o.c.
(double studs)	1 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	1 10d box (3 x .128)	1 16d com $(3^{1}/_{2} \times .162)$	1 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	1 16d com $(3^{1}/_{2} \times .162)$	1 12d com (3 <sup>1</sup> / <sub>4</sub> x .148)	1 8d com $(2^{1}/_{2} \times .131)$
not at braced walls	@ 8" o.c.		@ 16" o.c.	@ 16" o.c.	]	1 10d com (3 x .148)	1 3 <sup>1</sup> / <sub>4</sub> x .120
	1 3 x .131		1 3 x .131	1 3 x .131	]	1 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	1 3 x .120
See Figure 10A			1 10d box (3 x .128)	1 10d box (3 x .128)		$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	-
Stud-to-stud and			Connection 9	Connection 9		1 3 X . 131	
abutting studs at			@ 16" o.c.	@ 16" o.c.	@ 16" o.c.	@ 12" o.c.	@ 8" o.c.
intersecting wall			1 16d com $(3^{1}/_{2} \times .162)$	1 16d com $(3^{1}/_{2} \times .162)$	1 16d com $(3^{1}/_{2} \times .162)$	1 12d com $(3^{1}/_{4} \times .148)$	$1 3^{1}/_{4} \times .120$
corners			@ 12" o.c.	@ 12" o.c.		1 10d com (3 x .148)	1 3 x .120
at braced walls			1 3 x .131	1 3 x .131	1	1 16d box $(3^{1}/_{2} \times .135)$	
			1 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	1 16d box $(3^{1}/_{2} \times .135)$	ĺ	1 3 <sup>1</sup> / <sub>4</sub> x .131	
See Figures 10A and 10B				[ · ] · · · · · · (· · 2 · · · · · · )		1 3 x .131	
-	Connection 23	Connection 8	Connection 8	Connection 8			
Abutting studs at	@ 24" o.c.	@ 12" o.c.	@ 24" o.c.	@ 24" o.c.	@ 12" o.c.	@ 8" o.c.	
corners and	1 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	1 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	1 16d com $(3^{1}/_{2} \times .162)$	1 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	1 16d com $(3^{1}/_{2} \times .162)$	1 3 <sup>1</sup> / <sub>4</sub> x .131	
intersections not at braced walls	@ 16" o.c.		@ 16" o.c.	@ 16" o.c.	1 12d com $(3^{1}/_{4} \times .148)$	1 3 x .131	
not at braced wans	1 3 x .131		1 3 x .131	1 3 x .131	1 10d com (3 x .148)	1 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	
See Figure 10B			1 10d box (3 x .128)	1 10d box (3 x .128)	1 16d box $(3^{1}/_{2} \times .135)$	1 3 <sup>1</sup> / <sub>4</sub> x .120	
<b>3</b>		•				1 3 x .120	
	Connection 14	Connection 9	Connection 10	Connection 10		1	1
Built-up header	@16" o.c.	@16" o.c.	@16" o.c.	@16" o.c.	@12" o.c.	@8" o.c.	
2-by to 2-by	along each edge	along each edge	along each edge	along each edge	along each edge	along each edge	-
(with or without	1 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	1 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)		16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	1 16d com $(3^{1}/_{2} \times .162)$	1 12d com $(3^{1}/_{4} \times .148)$	
1/2" spacer)			<b>@12" o.c.</b> 1 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	<b>@12" o.c.</b> 1 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	1 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	1 10d com (3 x .148) 1 $3^{1}/_{4}$ x .131	-
			1 160 DOX (3 / <sub>2</sub> X . 135)	1 160 DOX (3 / <sub>2</sub> X . 135)	-	1 3 x .131	-
See Figure 10C						$1 3^{1}/_{4} \times .120$	-
						1 3 x .120	
	Connection 16	Connection 9	Connection 11	Connection 11		1 10 . 120	
Continuous header to	4 8d com $(2^{1}/_{2} \times .131)$	4 8d box $(2^{1}/_{2} \times .113)$	4 8d com $(2^{1}/_{2} \times .131)$	4 8d com (2½ x .131)	3 16d com $(3^{1}/_{2} \times .162)$	4 3 <sup>1</sup> / <sub>4</sub> x .131	5 3 <sup>1</sup> / <sub>4</sub> x .120
stud (toe-nail)	. 100 0011 (272 x 1101)	. 100 007 (2 12 7.110)	4 10d box (3 x .128)	4 10d box (3 x .128)	4 12d com $(3^{1}/_{4} \times .148)$	4 3 x 131	5 3 x .120
				5 8d box $(2^{1}/_{2} \times .113)$	4 10d com (3 x .148)	4 8d com $(2^{1}/_{2} \times .131)$	6 8d box $(2^{1}/_{2} \times .113)$
See Figure 10D					4 16d box $(3^{1}/_{2} \times .135)$		$6  2^{3}/_{8} \times .113$
	Connection 10a	Connection 13	Connection 12	Connection 12			
Double ter states t	@ 16" o.c.	@ 24" o.c.	@ 16" o.c.	@ 16" o.c.	@ 16" o.c.	@ 12" o.c.	@ 8" o.c.
Double top plates to each other	1 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	1 10d box (3 x .128)	1 16d com $(3^{1}/_{2} \times .162)$	1 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	1 16d com $(3^{1}/_{2} \times .162)$	1 12d com (3 <sup>1</sup> / <sub>4</sub> x .148)	1 8d com $(2^{1}/_{2} \times .131)$
each other	@ 12" o.c.		@ 12" o.c.	@ 12" o.c.		1 10d com (3 x .148)	1 3 <sup>1</sup> / <sub>4</sub> x .120
See Figure 10E	1 3 x .131		1 3 x .131	1 3 x .131		1 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	1 3 x .120
			1 10d box (3 x .128)	1 10d box (3 x .128)	Į	1 3 <sup>1</sup> / <sub>4</sub> x .131	
						1 3 x .131	

#### TABLE 10—FASTENING SCHEDULE-WALL FRAMING<sup>1</sup>

(continued)

	MINIMUN	I FASTENING REQUIRE	MENTS PRESCRIBED IN 1	THE CODE	ALTERNA	TIVE FASTENING REQUI	REMENTS
CONNECTION	2012 IBC	2012 IRC	2018 and 2015 IBC	2018 and 2015 IRC	Nails may be carbo	n steel or stainless steel,	except where noted
DESCRIPTION	Table 2304.9.1	Table R602.3(1)	Table 2304.10.1	Table R602.3(1)		otherwise.	
BEGORIA HOR	# Nail Size	# Nail Size	# Nail Size	# Nail Size	# Nail Size	# Nail Size	# Nail Size
	" (Type, inch)	" (Type, inch)	" (Type, inch)	(Type, inch)	" (Type, inch)	" (Type, inch)	" (Type, inch)
	Connection 10b	Connection 14	Connection 13	Connection 13 (13a for the 2015 IRC)			
<b>-</b>			n side of joint		Nails each	side of joint	
Top plate to top plate @ end joint (lap splice)	8 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	8 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	8 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	8 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)		12 16d box $(3^{1}/_{2} \times .135)$	
@ end joint (lap splice)	12 3 x .131		12 3 x .131	12 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	12 12d com (3 <sup>1</sup> / <sub>4</sub> x .148)		
See Figure 10F			12 10d box (3 x .128)	12 3 x .131	12 10d com (3 x .148)	12 3 x .131	]
Ŭ				12 10d box (3 x .128)	- · ·	· · ·	•
				Connection 13b (2015)			
				12 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	10 160 com $(3.72 \times .162)$	12 12d com (3 <sup>1</sup> / <sub>4</sub> x .148)	12 10d com (3 x .148
Top plate overlap at	Connection 13	Connection 19	Connection 18	Connection 17			
corners and	2 16d com $(3^{1}/_{2} \times .162)$	2 10d box (3 x .128)	2 16d com $(3^{1}/_{2} \times .162)$		2 16d com $(3^{1}/_{2} \times .162)$	3 3 <sup>1</sup> / <sub>4</sub> x .131	
intersections	3 3 x .131	J	3 3 x .131	3 3 x .131	3 12d com (3 <sup>1</sup> / <sub>4</sub> x .148)		
See Figure 10G			3 10d box (3 x .128)	3 10d box (3 x .128)	3 10d com (3 x .148)	4 3 <sup>1</sup> / <sub>4</sub> x .120	
See Figure 100	0				3 16d box $(3^{1}/_{2} \times .135)$	4 3 x .120	
Bottom plate to joist,	Connection 6a @ 16" o.c.	Connection 15 @ 16" o.c.	Connection 14 @ 16" o.c.	Connection 14 @ 16" o.c.	@ 16" o.c.	@ 12" o.c.	@ 8" o.c.
rim joist, band joist or	1 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	1 16d box $(3^{1}/_{2} \times .135)$		1 16d com $(3^{1}/_{2} \times .162)$		1 12 com (3 <sup>1</sup> / <sub>4</sub> x .148)	1 3 <sup>1</sup> / <sub>4</sub> x .120
blocking	@ 8" o.c.	1 100 DOX (3 /2 X . 135)	@ 12" o.c.	@ 12" o.c.		1 10d com (3 x .148)	1 3 x .120
not at braced walls	1 3 x .131	4	1 3 x .131	1 3 x .131	-	1 16d box $(3^{1}/_{2} \times .135)$	1 3 X . 120
	1 3 . 131	J	1 16d box $(3^{1}/_{2} \times .135)$	1 16d box $(3^{1}/_{2} \times .135)$	-	$1 3^{1}/_{4} \times .131$	
See Figure 10H			1 [100 b0x (3 /2 x .133)	1 [100 D0x (0 12 x 100)	-	1 3 x .131	
Bottom plate to joist,	Connection 6b	Connection 16	Connection 15	Connection 15			
rim joist, band joist or	@ 16" o.c.	@ 16" o.c.	@ 16" o.c.	@ 16" o.c.	@ 16" o.c.	@ 16" o.c.	@ 12" o.c.
blocking	3 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	3 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	2 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	2 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)		4 $3^{1}/_{4} \times .131$	2 16d com (3 <sup>1</sup> / <sub>2</sub> x .162
at braced walls	4 3 x .131		3 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	3 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	3 10d com (3 x .148)	4 3 x .131	, ,
		_	4 3 x .131	4 3 x .131	3 16d box $(3^{1}/_{2} \times .135)$	4 3 <sup>1</sup> / <sub>4</sub> x .120	
See Figure 10H						5 3 x .120	
Top or bottom plate to	Connections 7 and 8b	Connection 18	Connections 17 (2015) and 16b	Connection 16b			
stud (face/end nail)	2 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	2 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	2 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	2 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	3 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	4 3 <sup>1</sup> / <sub>4</sub> x .131	4 3 <sup>1</sup> / <sub>4</sub> x .120
	3 3 x .131		3 3 x .131	3 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	3 12d com (3 <sup>1</sup> / <sub>4</sub> x .148)		4 3 x .120
See Figure 10J			3 10d box (3 x .128)	3 3 x .131	3 10d com (3 x .148)	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	
				3 10d box (3 x .128)	3 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)		
	Connection 8	Connection 17	Connection 16a	Connection 16a			
Stud to top or bottom	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	3 8d box $(2^{1}/_{2} \times .113)$	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	3 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	3 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	4 3 <sup>1</sup> / <sub>4</sub> x .131	5 3 <sup>1</sup> / <sub>4</sub> x .120
Stud to top or bottom plate (toe nail)			4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 4 3 x .131	3 16d box (3 <sup>1</sup> / <sub>2</sub> x .135) 4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	4 12d com (3 <sup>1</sup> / <sub>4</sub> x .148)	4 3 x .131	5 3 x .120
plate (toe nail)	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	3 8d box $(2^{1}/_{2} \times .113)$	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	3 16d box (3 <sup>1</sup> / <sub>2</sub> x .135) 4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 4 3 x .131	4 12d com (3 <sup>1</sup> / <sub>4</sub> x .148) 4 10d com (3 x .148)		5 3 x .120 6 8d box (2 <sup>1</sup> / <sub>2</sub> x .113)
•	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	3 8d box $(2^{1}/_{2} \times .113)$	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 4 3 x .131	3 16d box (3 <sup>1</sup> / <sub>2</sub> x .135) 4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 4 3 x .131 4 10d box (3 x .128)	4 12d com (3 <sup>1</sup> / <sub>4</sub> x .148)	4 3 x .131	5 3 x .120 6 8d box (2 <sup>1</sup> / <sub>2</sub> x .113) 6 2 <sup>3</sup> / <sub>8</sub> x .113
plate (toe nail)	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	3 8d box $(2^{1}/_{2} \times .113)$	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 4 3 x .131 4 10d box (3 x .128) Connection 18	3 16d box (3 <sup>1</sup> / <sub>2</sub> x .135) 4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 4 3 x .131	4 12d com (3 <sup>1</sup> / <sub>4</sub> x .148) 4 10d com (3 x .148) 4 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	4 3 x .131	5       3 x .120         6       8d box (2 <sup>1</sup> / <sub>2</sub> x .113)         6       2 <sup>3</sup> / <sub>8</sub> x .113         6       6d com (2 x .113)
plate (toe nail) See Figure 10K	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 4 3 x .131 Connection 20	3         8d         box (2 <sup>1</sup> / <sub>2</sub> x .113)           2         16d         box (3 <sup>1</sup> / <sub>2</sub> x .135)           Connection 20	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 4 3 x .131 4 10d box (3 x .128) Connection 18 (19 for 2015 IBC)	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	4 12d com (3 <sup>1</sup> / <sub>4</sub> x .148) 4 10d com (3 x .148) 4 16d box (3 <sup>1</sup> / <sub>2</sub> x .135) Nails must	4 3 x .131 4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) be carbon steel, bright or	5       3 x .120         6       8d box (2 <sup>1</sup> / <sub>2</sub> x .113)         6       2 <sup>3</sup> / <sub>8</sub> x .113         6       6d com (2 x .113)         galvanized.
plate (toe nail)	4       8d com (2 <sup>1</sup> / <sub>2</sub> x .131)         4       3 x .131         Connection 20         2       8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	3 8d box (2 <sup>1</sup> / <sub>2</sub> x .113) 2 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 4 3 x .131 4 10d box (3 x .128) Connection 18 (19 for 2015 IBC) 2 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	4         12d com (3 <sup>1</sup> / <sub>4</sub> x .148)           4         10d com (3 x .148)           4         16d box (3 <sup>1</sup> / <sub>2</sub> x .135)           Nails must           2         16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	4 3 x .131 4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) <b>be carbon steel, bright or</b> 2 3 <sup>1</sup> / <sub>4</sub> x .131	5       3 x .120         6       8d box (2 <sup>1</sup> / <sub>2</sub> x .113)         6       2 <sup>3</sup> / <sub>8</sub> x .113         6       6d com (2 x .113)         galvanized.       3         3       3 <sup>1</sup> / <sub>4</sub> x .120
plate (toe nail) See Figure 10K 1" Diagonal brace to stud/plate (face-nail)	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 4 3 x .131 Connection 20	3         8d         box (2 <sup>1</sup> / <sub>2</sub> x .113)           2         16d         box (3 <sup>1</sup> / <sub>2</sub> x .135)           Connection 20	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 4 3 x .131 4 10d box (3 x .128) Connection 18 (19 for 2015 IBC) 2 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 2 3 x .131	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	4 3 x .131 4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) <b>be carbon steel, bright or</b> 2 3 <sup>1</sup> / <sub>4</sub> x .131 2 3 x .131	$\begin{array}{c} 5 & 3 \times .120 \\ \hline 6 & 8d \ box \ (2^{7}/_{2} \times .113) \\ \hline 6 & 2^{3}/_{6} \times .113 \\ \hline 6 & 6d \ com \ (2 \times .113) \\ \hline \\ \textbf{galvanized.} \\ \hline 3 & 3^{1}/_{4} \times .120 \\ \hline 3 & 3 \times .120 \\ \end{array}$
plate (toe nail) See Figure 10K 1" Diagonal brace to	4       8d com (2 <sup>1</sup> / <sub>2</sub> x .131)         4       3 x .131         Connection 20         2       8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	3         8d         box (2 <sup>1</sup> / <sub>2</sub> x .113)           2         16d         box (3 <sup>1</sup> / <sub>2</sub> x .135)           Connection 20	4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 4 3 x .131 4 10d box (3 x .128) Connection 18 (19 for 2015 IBC) 2 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	4         12d com (3 <sup>1</sup> / <sub>4</sub> x .148)           4         10d com (3 x .148)           4         16d box (3 <sup>1</sup> / <sub>2</sub> x .135)           Nails must           2         16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	4 3 x .131 4 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) <b>be carbon steel, bright or</b> 2 3 <sup>1</sup> / <sub>4</sub> x .131	5       3 x .120         6       8d box (2 <sup>1</sup> / <sub>2</sub> x .113)         6       2 <sup>3</sup> / <sub>8</sub> x .113         6       6d com (2 x .113)         galvanized.       3         3       3 <sup>1</sup> / <sub>4</sub> x .120

#### TABLE 10—FASTENING SCHEDULE–WALL FRAMING<sup>1</sup> (cont.)

For SI: 1 inch = 25.4 mm

<sup>1</sup>Alternative fastening requirements shown in this table have been evaluated as alternatives to the 2018, 2015 and 2012 IBC and IRC. They can be used under earlier editions of the IBC and IRC where the prescriptive fastening requirements are no worse than those shown in the table above for the 2018, 2015 and 2012 codes.

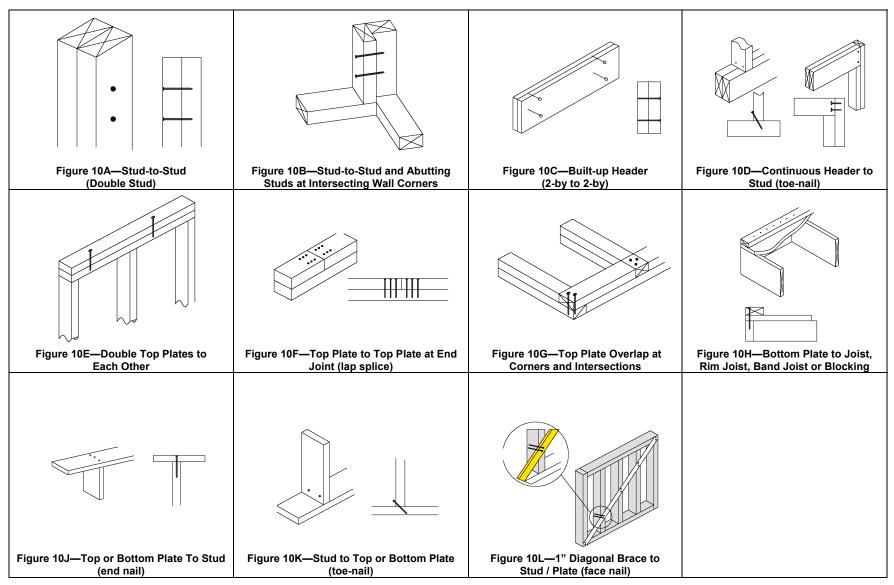


FIGURE 10-PRESCRIPTIVE FASTENING DETAILS - WALL FRAMING

	MINIMUN	I FASTENING REQUIRE	MENTS PRESCRIBED IN 1	HE CODE		ALTERNA	TIVE	FASTENING REQUI	REM	ENTS
	2012 IBC	2012 IRC	2018 and 2015 IBC	2018 and 2015 IRC		Nails may be carbor	n ste	el or stainless steel,	exce	pt where noted
CONNECTION DESCRIPTION	Table 2304.9.1	Table R602.3(1)	Table 2304.10.1	Table R602.3(1)				otherwise.		
DESCRIPTION	" Nail Size	# Nail Size	# Nail Size	# Nail Size	#	Nail Size	#	Nail Size	#	Nail Size
	# (Type, inch)	# (Type, inch)	# (Type, inch)	# (Type, inch)	#	(Type, inch)	#	(Type, inch)	#	(Type, inch)
Blocking between	Connection 11	Connection 1	Connection 1	Connection 1						
joists or rafter to top	Nails (total)	Nails (total)	Nails at each end	Nails (total)		·		Nails at each end		
plate (toe-nail)	3 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	3 8d box (2 <sup>1</sup> / <sub>2</sub> x .113)	3 8d com $(2^{1}/_{2} \times .131)$	3 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)		16d com (3 <sup>1</sup> / <sub>2</sub> x .162)				3 <sup>1</sup> / <sub>4</sub> x .120
See Figure 11A	3 3 x .131		3 3 x .131	3 3 x .131		$12d \text{ com } (3^{1}/_{4} \text{ x } .148)$		3 x .131	4	3 x .120
Occ rigule rint			3 10d box (3 x .128)	3 10d box (3 x .128)		10d com (3 x .148)	3	8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	5	8d box (2 <sup>1</sup> / <sub>2</sub> x .113)
	Orana stien 45	O anno ation O	O a mana attication O	4 8d box (2 <sup>1</sup> / <sub>2</sub> x .113)	3	16d box (3 <sup>1</sup> / <sub>2</sub> x .135)				i
Ceiling joist to plate (toe-nail)	Connection 15 3 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	Connection 2	Connection 2 3 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	Connection 2 3 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	2			arbon steel, bright or $3^{1}/_{4} \times .131$		anized. 8d box (2 <sup>1</sup> / <sub>2</sub> x .113)
nail thru each side	5 3 x .131	3 8d box (2 <sup>1</sup> / <sub>2</sub> x .113)	3 10d box (3 x .128)	3 10d box (3 x .128)		$12d \text{ com } (3^{1}/_{2} \text{ x} .162)$		3 /4 X . 13 1 3 X .131		$2^{3}/_{8} \times .113$
	5 5 2.151	1	3 3 x .131	3 3 x .131		$120 \text{ com} (37_4 \text{ x} . 148)$ 10d com (3 x .148)		$3^{1}/_{4} \times .120$		6d com (2 x .113)
See Figure 11B			5 5 . 151	5 5 8 . 151		$16d \text{ box } (3^{1}/_{2} \text{ x} .135)$		3 x .120	5	
Ceiling joist (not	Connection 17	Connection 3	Connection 3	Connection 3	5	100 00x (5 /2 x . 155)	-	5 . 120		
connected to parallel	3 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	3 10d box (3 x .128)	3 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	3 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	3	16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	Δ	16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	5	3 <sup>1</sup> / <sub>4</sub> x .120
		0 100 D0x (0 x .120)	4 3 x .131	4 3 x .131		$12d \operatorname{com} (3^{1}/_{4} \times .148)$		$3^{1}/_{4} \times .131$		3 x .120
over partition	4 0 X 101	1	4 10d box (3 x .128)	4 10d box (3 x .128)		10d com (3 x .148)		3 x .131	Ŭ	0 X . 120
See Figure 11C			1 100 000 (0 x 1120)	100 000 (0 x 1120)				0 X . 10 1		
	Connection 18		Connection 4	Connection 4						
Ceiling joist to parallel				See 2018 IRC Section						
rafters and at laps	See IBC Section		See IBC Section	R802.5.2 and Table						
	2308.1.4.1 and Table		2308.7.3.1 and Table	R802.5.2 (2015 IRC						
See Figure 11D	2308.1.4.1		2308.7.3.1	Section R802.3.1, R802.3.2 & Table						
				R802.5.1(9)						
	Connection 26	Connection 4	Connection 5	Connection 5						
Collar tie to rafter	3 10d com (3 x .148)	3 10d box (3 x .128)	3 10d com (3 x .148)	3 10d com (3 x .148)	3	16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	4	3 <sup>1</sup> / <sub>4</sub> x .131	5	3 <sup>1</sup> / <sub>4</sub> x .120
	4 3 x .131		4 10d box (3 x .128)	4 10d box (3 x .128)		12d com (3 <sup>1</sup> / <sub>4</sub> x .148)		3 x .131		3 x .120
See Figure 11E		4	4 3 x .131	4 3 x .131		10d com (3 x .148)	5	8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	6	8d box $(2^{1}/_{2} \times .113)$
-			L	ь — ь	4	16d box (3 <sup>1</sup> / <sub>2</sub> x .135)				<b>.</b>
	Connection 19	Connection 5	Connection 6	Connection 6		Nails must b	be ca	arbon steel, bright or	galv	anized.
	3 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	3 10d com (3 x .148)	3 10d com (3 x .148)	3 10d com (3 x .148)	3	16d com (3 <sup>1</sup> / <sub>2</sub> x .162)		3 <sup>1</sup> / <sub>4</sub> x .131		3 <sup>1</sup> / <sub>4</sub> x .120
Roof rafter to plate	3 3 x .131	3 16d box $(3^{1}/_{2} \times .135)$	3 16d box $(3^{1}/_{2} \times .135)$	3 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	3	12d com (3 <sup>1</sup> / <sub>4</sub> x .148)		3 x .131	4	3 x .120
(toe-nail)	+ connectors per IBC		4 3 x .131	4 3 x .131		10d com (3 x .148)	4	8d com (2 <sup>1</sup> / <sub>2</sub> x .131)		-
	Section 2308.10.1		4 10d box (3 x .128)	4 10d box (3 x .128)	3	16d box (3 <sup>1</sup> / <sub>2</sub> x .135)				
See Figure 11F			+ connectors per IBC				+	connectors per IBC		
		1	Section 2308.7.5							
Ridge beam	Connection 28b	Connection 6	Connection 7a	Connection 7b	-			01/ 404	-	
	2 16d com $(3^{1}/_{2} \times .162)$	4 16d box $(3^{1}/_{2} \times .135)$	2 16d com $(3^{1}/_{2} \times .162)$	2 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)		$16d \text{ com } (3^{1}/_{2} \text{ x .162})$ 16d com $(3^{1}/_{2} \text{ x .162})$		3 <sup>1</sup> / <sub>4</sub> x .131		
(face/end nail)	( 2 )					100000013768162	- 4	3 x .131		
(face/end nail)	3 3 x .131		3 3 x .131	3 16d box $(3^{1}/_{2} \times .135)$	4			$2^{1}/\sqrt{120}$		
	( 2 )			3 3 x .131	4	10d com (3 x .148)	5	3 <sup>1</sup> / <sub>4</sub> x .120		
(face/end nail)	3 3 x .131		3 3 x .131 3 10d box (3 x .128)	3 3 x .131 3 10d box (3 x .128)	4		5	3 <sup>1</sup> / <sub>4</sub> x .120 3 x .120		
(face/end nail)	3 3 x .131	Connection 6	3 3 x .131 3 10d box (3 x .128) Connection 7b	3 3 x .131 3 10d box (3 x .128) Connection 7a	4	10d com (3 x .148) 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	5 5	3 x .120	6	8d box $(2^{1/2} \times 113)$
(face/end nail) See Figure 11G Roof rafter to 2-by ridge beam	3 3 x .131 Connection 28a 2 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	Connection 6	3         3 x .131           3         10d box (3 x .128)           Connection 7b           3         10d com (3 x .148)	3         3 x .131           3         10d box (3 x .128)           Connection 7a           3         10d com (3 x .148)	4 4 3	10d com (3 x .148) 16d box ( $3^{1}/_{2}$ x .135) 16d com ( $3^{1}/_{2}$ x .162)	5 5 5	3 x .120 3 <sup>1</sup> / <sub>4</sub> x .131	6	8d box (2 <sup>1</sup> / <sub>2</sub> x .113) 2 <sup>3</sup> / <sub>2</sub> x .113
(face/end nail) See Figure 11G Roof rafter to 2-by	3 3 x .131	Connection 6	3         3 x .131           3         10d box (3 x .128)           Connection 7b           3         10d com (3 x .148)           3         16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	3         3 x .131           3         10d box (3 x .128)           Connection 7a           3         10d com (3 x .148)           4         16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	4 4 3 4	$\frac{10d \text{ com } (3 \text{ x} .148)}{16d \text{ box } (3^{1}/_2 \text{ x} .135)}$ $\frac{16d \text{ com } (3^{1}/_2 \text{ x} .162)}{12d \text{ com } (3^{1}/_4 \text{ x} .148)}$	5 5 5 5	3 x .120 3 <sup>1</sup> / <sub>4</sub> x .131 3 x .131	6	2 <sup>3</sup> / <sub>8</sub> x .113
(face/end nail) See Figure 11G Roof rafter to 2-by ridge beam	3 3 x .131 Connection 28a 2 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	Connection 6	3         3 x .131           3         10d box (3 x .128)           Connection 7b           3         10d com (3 x .148)	3         3 x .131           3         10d box (3 x .128)           Connection 7a           3         10d com (3 x .148)	4 4 3 4 4	10d com (3 x .148) 16d box ( $3^{1}/_{2}$ x .135) 16d com ( $3^{1}/_{2}$ x .162)	5 5 5 5 5	3 x .120 3 <sup>1</sup> / <sub>4</sub> x .131		8d box (2 <sup>1</sup> / <sub>2</sub> x .113) 2 <sup>3</sup> / <sub>8</sub> x .113 6d com (2 x .113)

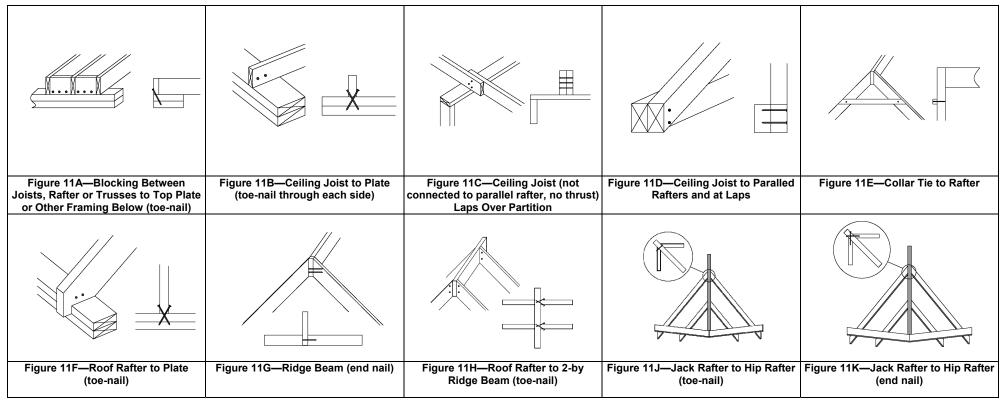
#### TABLE 11—FASTENING SCHEDULE\_CEILING AND ROOF FRAMING<sup>1</sup>

	MINIMUN	I FASTENING REQUIRE	IENTS PRESCRIBED IN T	ALTERNATIVE FASTENING REQUIREMENTS					
CONNECTION	2012 IBC	2012 IRC	2018 and 2015 IBC 2018 and 2015 IRC			Nails may be carbon s	,	ехсер	t where noted
DESCRIPTION	Table 2304.9.1 Table R602.3(1)		Table 2304.10.1	Table R602.3(1)			otherwise.		
	# Nail Size (Type, inch)	# Nail Size (Type, inch)	# Nail Size (Type, inch)	# Nail Size (Type, inch)	#	Nail Size (Type, inch)	Nail Size (Type, inch)	#	Nail Size (Type, inch)
	Connection 27a	Connection 6	Connection 7b	Connection 7a					
Jack rafter to hip	3 10d com (3 x .148)	4 16d box $(3^{1}/_{2} \times .135)$	3 10d com (3 x .148)	3 10d com (3 x .148)	3	16d com (3 <sup>1</sup> / <sub>2</sub> x .162) 5	3 <sup>1</sup> / <sub>4</sub> x .131		
(toe nail)	4 3 x .131		3 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	4 16d box $(3^{1}/_{2} \times .135)$	4	12d com (3 <sup>1</sup> / <sub>4</sub> x .148) 5	3 x .131		
See Figure 11J			4 3 x .131	4 3 x .131	4	10d com (3 x .148) 5	8d com (2 <sup>1</sup> / <sub>2</sub> x .131)		
occ rigule rib			4 10d box (3 x .128)	4 10d box (3 x .128)	4	16d box (3 <sup>1</sup> / <sub>2</sub> x .135)			
lask Defter to Llin	Connection 27b	Connection 6	Connection 7a	Connection 7b					
Jack Rafter to Hip	2 16d com $(3^{1}/_{2} \times .162)$	4 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	2 16d com $(3^{1}/_{2} \times .162)$	2 16d com $(3^{1}/_{2} \times .162)$	3	16d com (3 <sup>1</sup> / <sub>2</sub> x .162)			
(face/end nail)	3 3 x .131		3 3 x .131	3 16d box $(3^{1}/_{2} \times .135)$	4	12d com (3 <sup>1</sup> / <sub>4</sub> x .148)			
See Figure 11K			3 10d box (3 x .128)	3 3 x .131	4	10d com (3 x .148)			
				3 10d box (3 x .128)	4	16d box (3 <sup>1</sup> / <sub>2</sub> x .135)			

#### TABLE 11—FASTENING SCHEDULE-CEILING AND ROOF FRAMING<sup>1</sup> (cont.)

For SI: 1 inch = 25.4 mm

<sup>1</sup>Alternative fastening requirements shown in this table have been evaluated as alternatives to the 2018, 2015 and 2012 IBC and IRC. They can be used under earlier editions of the IBC and IRC where the prescriptive fastening requirements are no worse than those shown in the table above for the 2018, 2015 and 2012 codes.



	MINIMUM	EASTENING REQUIRE	MENTS PRESCRIBED IN 1			TIVE FASTENING REQUI	DEMENTO
	2012 IBC	2012 IRC	2018 and 2015 IBC	2018 and 2015 IRC		in steel or stainless steel,	-
CONNECTION	Table 2304.9.1	Table R602.3(1)	Table 2304.10.1	Table R602.3(1)	Nalis Illay be carbo	otherwise.	except where noted
DESCRIPTION	Noil Size	Nail Sizo	Noil Cine	N 11 O	" Nail Size		" Nail Size
	# (Type, inch)	# (Type, inch)	# (Type, inch)	# (Type, inch)	# (Type, inch)	# Nail Size (Type, inch)	# (Type, inch)
Joist to sill or girder	Connection 1	Connection 24	Connection 21 (22 for the 2015 IBC)	Connection 21			
(toe-nail) nail thru each side	3 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	3 8d box (2 <sup>1</sup> / <sub>2</sub> x .113)	3 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	3 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	3 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	3 3 <sup>1</sup> / <sub>4</sub> x .131	4 3 x .120
naii thru each side	3 3 x .131	]	3 3 x .131	3 3 x .131	3 12d com (3 <sup>1</sup> / <sub>4</sub> x .148)		5 8d box (2 <sup>1</sup> / <sub>2</sub> x .113)
See Figure 12A			3 10d box (3 x .128)	3 10d box (3 x .128)	3 10d com (3 x .148)	3 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	5 2 <sup>3</sup> / <sub>8</sub> x .113
, , , , , , , , , , , , , , , , , , ,		1		4 8d box (2 <sup>1</sup> / <sub>2</sub> x .113)	3 16d box $(3^{1}/_{2} \times .135)$	4 3 <sup>1</sup> / <sub>4</sub> x .120	5 6d com (2 x .113)
	Connection 12	Connection 25	Connection 22 (23 for the 2015 IBC)	Connection 22			•
Rim joist to top plate	6" o.c.	6" o.c.	6" o.c.	4" o.c.	@ 6" o.c	@ 6" o.c.	@ 4" o.c.
(toe-nail)	1 8d com $(2^{1}/_{2} \times .131)$	1 8d box (2 <sup>1</sup> / <sub>2</sub> x .113)	1 8d com $(2^{1}/_{2} \times .131)$	1 8d box $(2^{1}/_{2} \times .113)$	1 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)		1 8d box $(2^{1}/_{2} \times .113)$
	1 3 x .131		1 3 x .131	6" o.c.	1 12d com $(3^{1}/_{4} \times .148)$		1 $2^{3}/_{8} \times .113$
See Figure 12B			1 10d box (3 x .128)	1 8d com $(2^{1}/_{2} \times .131)$	1 10d com (3 x .148)	@ 4" o.c.	@ 3" o.c.
				1 3 x .131	1 16d box $(3^{1}/_{2} \times .135)$	1 $3^{1}/_{4}$ x .120	1 2 x .113
		1		1 10d box (3 x .128)	1 3 <sup>1</sup> / <sub>4</sub> x .131	1 3 x .120	1 2 <sup>1</sup> / <sub>4</sub> x .099
Joist to band joist (face/end nail)	Connection 29		Connection 28 (29 for the 2015 IBC)	Connection 26			
(lace/clid fiail)	3 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)		3 16d com $(3^{1}/_{2} \times .162)$	3 16d com $(3^{1}/_{2} \times .162)$	3 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)		6 $3^{1}/_{4} \times .120$
See Figure 12C	4 3 x .131		4 3 x .131	4 3 x .131	4 12d com (3 <sup>1</sup> / <sub>4</sub> x .148)		6 3 x .120
0		1	4 10d box (3 x .128)	4 10d box (3 x .128)	4 10d com (3 x .148)	4 3 x .131	
	Connection 24	Connection 30	Connection 26 (27 for the 2015 IBC)	Connection 27			
			staggered on opposite s			and bottom staggered on	
	@ 32" o.c.	@ 32" o.c.	@ 32" o.c	@ 32" o.c	@ 32" o.c.	@ 24" o.c.	@ 16" o.c.
Built up girder or beam	1 20d com (4 x .192)	1 10d box (3 x .128)	1 20d com (4 x .192)	1 20d com (4 x .192)	1 20d com (4 x .192)	1 10d com (3 x .148)	1 $3^{1}/_{4}$ x .120
Dant ap giraor or boain	@ 24" o.c.	•	@ 24" o.c	@ 24" o.c	@ 24" o.c.	1 16d box $(3^{1}/_{2} \times .135)$	1 3 x .120
See Figure 12D	1 3 x .131	1	1 3 x .131	1 3 x .131 1 10d box (3 x .128)	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$1 3^{1}/_{4} \times .131$	-
-		AND of each and	1 10d box (3 x .128)	1 10d box (3 x .128)			a a cille
	2 20d com (4 x 0.192)	AND at each end 2 10d box (3 x .128)	or splice (face nail): 2 20d com (4 x .192)	2 20d com (4 x .192)	2 20d com (4 x .192)	at each end or splice (face 3 10d com (3 x 148)	3 3 x .131
	3 3 x .131	2 100 DOX (3 X . 128)	2 200 com (4 x .192) 3 3 x .131	2 200 com (4 x .192) 3 3 x .131	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		$3 3^{1}/_{4} \times 120$
	5 5 7.151	1	3 10d box (3 x .128)	3 10d box (3 x .128)	3 12d com $(3^{1}/_{4} \times .148)$		4 3 x .120
Ledger strip	Connection 30	Connection 31	Connection 27 (28 for the 2015 IBC)	Connection 28		0 0 14 A 101	. 0
(face nail)	3 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	3 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)		3 16d com $(3^{1}/_{2} \times .162)$	3 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)	5 3 <sup>1</sup> / <sub>4</sub> x .131	6 3 <sup>1</sup> / <sub>4</sub> x .120
(	4 3 x .131	(2.2.1.100)	4 3 x .131	4 16d box $(3^{1}/_{2} \times .135)$	4 12d com $(3^{1}/_{4} \times .148)$		6 3 x .120
See Figure 12E		4	4 10d box (3 x .128)	4 3 x .131	4 10d com (3 x .148)	6 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	
				4 10d box (3 x .128)	4 16d box $(3^{1}/_{2} \times .135)$		
	Connection 2		Connection 29 (30 for the 2015 IBC)	Connection 29			
Bridging to Joist	Nails (total)		Nails at each end	Nails at each end		Nails at each end	
(toe nail)	2 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)		2 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	2 10d box (3 x .128)	2 16d com (3 <sup>1</sup> / <sub>2</sub> x .162)		3 8d box (2 <sup>1</sup> / <sub>2</sub> x .113)
I	2 3 x .131		2 3 x .131	(2018 IRC only)	2 12d com (3 <sup>1</sup> / <sub>4</sub> x .148)		3 2 <sup>3</sup> / <sub>8</sub> x .113
See Figure 12F		1	2 10d box (3 x .128)	2 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	2 10d com (3 x .148)	2 8d com (2 <sup>1</sup> / <sub>2</sub> x .131)	3 6d com (2 x .113)
See Figure 12F	- 1	1	2 10d box (3 x .128)	2 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 2 3 x .131	2 10d com (3 x .148) 2 16d box (3 <sup>1</sup> / <sub>2</sub> x .135)	2 8d com (2 <sup>1</sup> / <sub>2</sub> x .131) 3 3 <sup>1</sup> / <sub>4</sub> x .120 3 3 x .120	3 6d com (2 x .113) 4 2 <sup>1</sup> / <sub>4</sub> x .099

#### TABLE 12—FASTENING SCHEDULE-FLOOR FRAMING<sup>1</sup>

For **SI:** 1 inch = 25.4 mm

<sup>1</sup>Alternative fastening requirements shown in this table have been evaluated as alternatives to the 2018, 2015 and 2012 IBC and IRC. They can be used under earlier editions of the IBC and IRC where the prescriptive fastening requirements are no worse than those shown in the table above for the 2018, 2015 and 2012 codes.

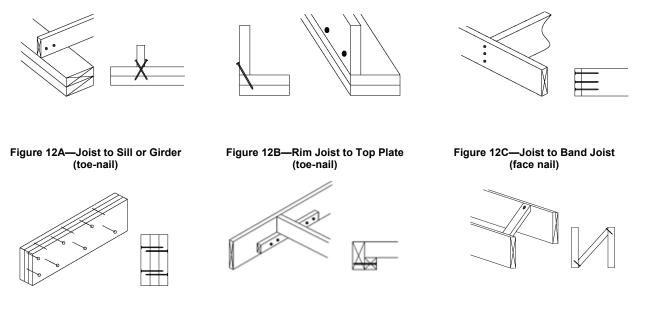


Figure 12D—Built-up Girder or Beam staggerd on both sides

Figure 12E—Ledger Strip (face nail)

Figure 12F—Bridging to Joist (toe-nail)

FIGURE 12—PRESCRIPTIVE FASTENING DETAILS – FLOOR FRAMING

#### TABLE 13—SUMMARY OF ALTERNATIVE FASTENING DESIGNS RECOGNIZED IN TABLES 10 THROUGH 12<sup>1,2,3,4</sup>

				I	NAIL SIZ	E (DIAM	ETER X	LENGTH	) (inches	5)			
CONNECTION	3 <sup>1</sup> / <sub>2</sub> x 0.162	3 <sup>1</sup> / <sub>4</sub> x 0.148	3 x 0.148	3 <sup>1</sup> / <sub>2</sub> x 0.135	3 <sup>1</sup> / <sub>4</sub> x 0.131	3 x 0.131	2 <sup>1</sup> / <sub>2</sub> x 0.131	3 <sup>1</sup> / <sub>4</sub> x 0.120	3 x 0.120	2 <sup>1</sup> / <sub>2</sub> x 0.113	2 <sup>3</sup> / <sub>8</sub> x 0.113	2 x 0.113	2 <sup>1</sup> / <sub>4</sub> x 0.099
Double studs (face nail)	24"	16"	16"	16"	all Frami 16"	ng 16"	8"	8"	8"				
Typical	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.				
At braced walls	16"	12"	12"	12"	12"	12"		8"	8"				
	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.		0.C.	0.C.				
Abutting studs at corners and	12"	12"	12"	12"	8"	8"	8"	8"	8"				
intersections	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.				
Typical At braced walls	12"	12"	12"	12"	12"	12"		8"	8"		-		
At braced wails	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.		0.C.	0.C.				
Built up header 2" to 2" w/ 1/2"	12"	8"	8"	12"	8"	8"		8"	8"				
spacer	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.		0.C.	0.C.				
Continuous header to stud (toe nail)	3	4	4	4	4	4	4	5	5	6	6		
Double top plates to each other	16"	12"	12"	12"	12"	12"	8"	8"	8"				
(face nail)	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.				
Top plate to top plate at end													
joint (lap splice)	8	12	12	12	12	12							
(each side of joint)													
For 2015 IRC Connection 13b	10	12	12										
Top plate overlap at corners and intersections (face nail)	2	3	3	3	3	3		4	4				
Sole plate to joist or blocking not	16"	12"	12"	12"	12"	12"		8"	8"		-		
at braced wall panels	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.		0.C.	0.C.				
Sole Plate to joist or blocking at	2@	3@	3@	3@	4@	4@		4@	5@				
braced wall panel	16"	16"	16"	16"	16"	16"		16"	16"				
<b>-</b>	0.C.	0.C.	0.C.	0.C.	0.C.	0.C.		0.C.	0.C.				
Top or sole plate to stud (end nail)	3	3	3	3	4	4	4	4	4				
Stud to top or sole plate (toe- nail)	3	4	4	4	4	4	4	5	5	6	6	6	
Diagonal bracing to stud/plate <sup>5</sup>	2	2	2	2	2	2	2	3	3	3	3		4
	1			Ceiling a	and Roof	Framing	g						
Blocking Between Joists or													
Rafter to Top Plate (toe-nail) (each end)	3	3	3	3	3	3	3	4	4	5			
Ceiling joist to plate <sup>5</sup>	3	3	3	3	3	3	3	4	4	5	5	5	
Ceiling joists laps over partitions					-		Ű			Ű		Ű	
(no thrust)	3	4	4	4	4	4		5	5				
Ceiling joist to parallel rafter							See Ta	able 11		-	-	-	-
Collar tie to rafter	3	3	3	4	4	4	5	5	5	6			
Roof rafter to plate (toe-nail) (+ connectors per IBC) <sup>5</sup>	3	3	3	3	4	4	4	4	4				
Roof rafter to 2-by ridge beam	3	4	4	4	4	4		5	5				
(end-nail rafter to beam) Roof rafter to 2-by ridge beam				<u> </u>	<u> </u>				<u> </u>				
(toe-nail rafter to beam)	3	4	4	4	5	5	5	6	6	6	6	6	
Jack rafter to hip (toe-nail)	3	4	4	4	5	5	5						
Jack rafter to hip (end nail)	3	4	4	4									
					or Fram								
Joist to sill or girder (toe-nail)	3	3	3	3	3	3	3	4	4	5	5	5	
Rim joist to top plate(Toe-nail)	6"	6"	6"	6"	6"	6"	6"	4"	4"	4"	4"	3"	3"
Joist to band Joist (face nail)	0.C. 3	0.C. 4	0.C. 4	0.C. 4	0.C. 4	0.C. 4	0.C.	0.C. 6	0.C. 6	0.C.	0.C.	0.C.	0.C.
Built-up girders & beams	3 24"	4 24"	4 24"	4 24"	4 24"	4 24"		16"	16"				
Face-nail @ top and bottom	24 0.C.	24 0.C.	0.C.	24 0.C.	0.C.	0.C.		0.C.	0.C.				
PLUS # at ends or splice	3	3	3	3	3	3		3	4				
Ledger Strip	3	4	4	4	5	5	6	6	6				
Bridging to Joist (toe-nail)	2	2	2	2	2	2	2	3	3	3	3	3	4

For **SI:** 1 inch = 25.4 mm

<sup>1</sup>Alternative fastening requirements shown in this table have been evaluated as alternatives to the 2018, 2015 and 2012 IBC and the 2018, 2015 and 2012 IRC. They can be used under earlier editions of the IBC and IRC where the prescriptive fastening requirements are no worse than those shown in the Tables 10 through 12.

<sup>2</sup>This fastening schedule applies to framing members having an actual thickness of 1<sup>1</sup>/<sub>2</sub>" (nominal "2-by" lumber).

<sup>3</sup>Fastening schedule only applies to buildings of conventional wood frame construction where wind or seismic analysis is not required by the applicable code. In areas where wind or seismic analysis is required, required fastening must be determined by structural analysis.

<sup>4</sup>Nails may be carbon steel (bright or galvanized) or stainless steel, unless otherwise noted.

<sup>5</sup>Nails must be carbon steel (bright or galvanized).

#### APPENDIX A—REFERENCE DESIGN INFORMATION

#### A1.0 Reference Design Values for Nailed Connections:

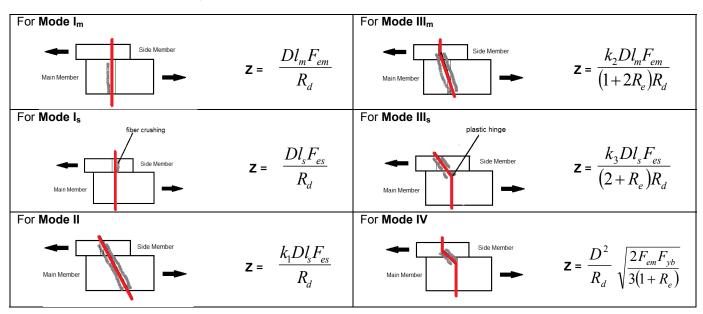
The information in this section is applicable to the nails recognized in Appendix B of this report.

#### A1.1 Source:

The equations shown here for nailed connections are found in the 2018, 2015, 2012 and 2005 ANSI/AWC National Design Specification (NDS) for Wood Construction.

#### A1.2 Reference Lateral Design Values:

Reference lateral design values are based on the yielding of connections as wood fibers are crushed and/or fastener shanks are bent. Reference lateral design values are determined from the lowest resulting value from six yield limit equations. These equations and depictions of these yield modes are shown below:



where:

$$\kappa_{1} = \frac{\sqrt{R_{e} + 2R_{e}^{2}(1 + R_{t} + R_{t}^{2}) + R_{t}^{2}R_{e}^{3}} - R_{e}(1 + R_{t})}{(1 + R_{e})}$$

$$K_2 = -1 + \sqrt{2(1+R_e) + \frac{2F_{yb}(1+2R_e)D^2}{3F_{ea}l_m^2}}$$

$$K_3 = -1 + \sqrt{\frac{2(1+R_e)}{R_e} + \frac{2F_{yb}(2+R_e)D^2}{3F_{em}l_s^2}}$$

- = Reference lateral design value, lbf
  - = F<sub>em</sub> / F<sub>es</sub>

Z R<sub>e</sub>

I<sub>m</sub> Is F<sub>em</sub>

- = Length of nail in main member (member holding point), inches
- = Length of nail in side member, inches
- Dowel bearing strength of main member (member holding point), psi [See 2018 and 2015 NDS Table 12.3.3 (2012 NDS Table 11.3.3, 2005 NDS Table 11.3.2)]
- *F<sub>es</sub>* = Dowel bearing strength of side member, psi [See 2018 and 2015 NDS Table 12.3.3 (2012 NDS Table 11.3.3, 2005 NDS Table 11.3.2)]
  - = Bending yield strength of nail, psi (see Appendix B)
  - = Nominal nail diameter, inch (see Appendix B)
  - = 2.2 for D  $\leq 0.17$ ", 10D +0.5 for 0.17 < D < 0.25
- $F_{yb} = \text{Bendin}$  D = Nomin  $R_d = 2.2 \text{ for}$  $R_t = I_m / I_s$

#### A1.3 Reference Withdrawal Design Values:

#### A1.3.1 Smooth or Deformed Shank, Carbon Steel (Bright or Galvanized) Nails:

The reference withdrawal design value per unit length of penetration of a smooth or deformed shank, carbon steel nail driven into the side grain (perpendicular to the fiber) of the wood is calculated as follows:

$$W = 1380 \ G^{5/2} D$$
 (Eq. A1.3.1)

Where:

- W = Nail reference withdrawal design value in pounds-force per lineal inch of penetration into the member holding the nail point.
- D = Nominal diameter of the nail shank in inches, for  $0.092 \le D \le 0.375$ .
- *G* = The assigned specific gravity of the wood found in Table A or the tables indicated below, as applicable:

For **SI**: 
$$W = 9.515 \ G^{5/2} D$$
 (Eq. A1.3.2)

Where:

- W = Nail reference withdrawal design value in Newtons per lineal millimeter of penetration into the member holding the nail point.
- D = Nominal diameter of the nail shank in millimeters, for 2.33  $\leq$  D  $\leq$  9.525.
- *G* = The assigned specific gravity of the wood found in Table A or the tables indicated below, as applicable:

Code	Sawn Lumber	Wood Structural Panels
2018 and 2015 IBC	2018 and 2015 NDS Table 12.3.3A	2018 and 2015 NDS Table 12.3.3B
2012 IBC	2012 NDS Table 11.3.3A	2012 NDS Table 11.3.3B
2009 and 2006 IBC	2005 NDS Table 11.3.2A	2005 NDS Table 11.3.2B

#### A1.3.2 Smooth or Deformed Shank, Stainless Steel Nails:

The reference withdrawal design value per unit length of penetration of a smooth or deformed shank, stainless steel nail driven into the side grain (perpendicular to the fiber) of the wood is calculated as follows:

$$W = 465 \ G^{3/2} D$$
 (Eq. A1.3.3)

Where:

- W = Nail reference withdrawal design value in pounds-force per lineal inch of penetration into the member holding the nail point.
- D = Nominal diameter of the nail shank in inches, for  $0.092 \le D \le 0.375$ .
- G = The assigned specific gravity of the wood found in Table A or the tables referenced in Section A1.3.1, as applicable.

For **SI**: 
$$W = 3.206 \ G^{3/2} D$$
 (Eq. A1.3.4)

Where:

- W = Nail reference withdrawal design value in Newtons per lineal millimeter of penetration into the member holding the nail point.
- D = Nominal diameter of the nail shank in millimeters, for 2.33  $\leq$  D  $\leq$  9.525.
- G = The assigned specific gravity of the wood found in Table A or the tables referenced in Section A1.3.1, as applicable.

#### TABLE A—ASSIGNED SPECIFIC GRAVITY AND DOWEL BEARING STRENGTH FOR SELECT WOOD SPECIES

	SPECIFIC	DOWEL-BEARING	STRENGTH F <sub>e</sub> (psi)
SPECIES	<b>GRAVITY</b> <sup>1</sup>	Nailed Connections	Stapled Connections
Aspen	0.39	2,950	3,850
Balsam Fir	0.36	2,550	3,450
Beech-birch-hickory	0.71	8,850	9,750
Coast Sitka Spruce	0.39	2,950	3,850
Douglas Fir-larch	0.50	4,650	5,550
Douglas Fir-south	0.46	4,000	4,900
Eastern Hemlock	0.41	3,200	4,100
Eastern Hemlock-tamarack	0.41	3,200	4,100
Eastern Hemlock-tamarack (north)	0.47	4,150	5,050
Eastern softwoods	0.36	2,550	3,450
Eastern Spruce	0.41	3,200	4,100
Eastern White Pine	0.36	2,550	3,450
Hem-Fir	0.43	3,500	4,400
Mountain Hemlock	0.47	4,150	5,050
Northern Pine	0.42	3,350	4,250
Northern Species	0.35	2,400	3,300
Northern White Cedar	0.31	1,900	2,800
Ponderosa Pine	0.43	3,500	4,400
Red Oak	0.67	7,950	8,850
Red Pine	0.44	3,650	4,550
Sitka Spruce	0.43	3,500	4,400
Southern Pine	0.55	5,550	6,450
Spruce-Pine-Fir	0.42	3,350	4,250
Western Cedars	0.36	2,550	3,450
Western Cedars (North)	0.35	2,400	3,300
Western Hemlock	0.47	4,150	5,050
Western White Pine	0.40	3,100	4,000
White Oak	0.73	9,300	10,200
Yellow Poplar	0.43	3,500	4,400
	WOOD STRU	CTURAL PANELS	
Plywood: Structural 1, Marine	0.50	4,650	5,550
Plywood: Other Grades	0.42	3,350	4,250
Oriented Strand Board All Grades	0.50	4,650	5,550

For **SI:** 1 psi = 6.89 kN/m<sup>2</sup>.

<sup>1</sup>Specific gravity based on weight and volume when oven dry.

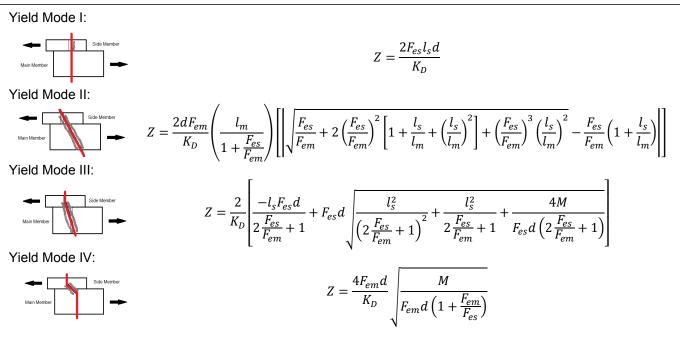
### A2.0 Reference Design Values for Stapled Connections:

#### A2.1 Source:

The equations shown here for stapled connections are found in the ICC-ES Acceptance Criteria for Staples (AC201) dated March 2018.

#### A2.2 Reference Lateral Design Values:

Reference lateral design values for stapled connections must be determined using the minimum result from the equations shown below. These equations are relevant to wood-to-wood connections and to connections in which steel sheet metal is stapled to wood. The steel side member shall have sufficient thickness to prevent tearing of the steel sheet when loaded. Determination of dowel bearing strength of the sheet metal must consider Section I.2 of Appendix I of the NDS. Reference lateral design values are for normal load duration and must be multiplied by all applicable adjustment factors in accordance with the NDS.



where:

- Z = Reference lateral design value for staple (2 legs), lbf.
- $F_{em}$  = Dowel bearing strength of the main member, psi = 900 psi +  $F_e$  from 2018 and 2015 NDS Table 12.3.3 (2012 NDS Table 11.3.3, 2005 NDS Table 11.3.2) for D<<sup>1</sup>/<sub>4</sub>"
- $F_{es}$  = Dowel bearing strength of the side member, psi = 900 psi +  $F_e$  from 2018 and 2015 NDS Table 12.3.3 (2012 NDS Table 11.3.3, 2005 NDS Table 11.3.2) for D<<sup>1</sup>/<sub>4</sub>"
- d = Nominal wire diameter, inch, from Table 3.2.
- M = Minimum staple bending moment, in-lbs, from Table 3.2.
- $I_s$  = Length of staple in side member, inches.
- $I_m$  = Length of staple in main member, inches, (minimum of 12D, where D is the nominal wire diameter from Table 3.2).
- $K_D$  = Diameter coefficient for staple connections = 2.2

#### A2.3 Reference Withdrawal Design Values:

The reference withdrawal design value per unit length of penetration of staples driven into the side grain (perpendicular to the fiber) of the wood is calculated as follows:

$$W = 2760 G^{5/2} D$$
 (Eq. A1.3.5)

where:

- W = Staple reference withdrawal design value, in pounds-force per lineal inch of penetration into the member holding both staple legs.
- G = the assigned specific gravity of the wood (see Table A and Section A1.3.1).
- D = Nominal wire diameter, in inches, from Table 3.2.

For **SI**: 
$$W = 19.03 \ G^{5/2} D$$
 (Eq. A1.3.6)

where:

- W = Staple reference withdrawal design value, in Newtons per linear millimeter of penetration into the member holding both staple legs.
- G = The assigned specific gravity of the wood (see Table A and Section A1.3.1).
- D = Nominal wire diameter, in millimeters, from Table 3.2.

#### A3.0 DESIGN INFORMATION FOR DEFLECTION CALCULATIONS FOR DIAPHRAGMS AND SHEAR WALLS

#### A3.1 NAILS:

To determine the deflection of diaphragms and shear walls constructed as described in Tables 5 through 9, refer to Sections 4.2.2 and 4.3.2 of the AWC Special Design Provisions for Wind and Seismic (SDPWS), respectively. For 0.120 inch nails, use the  $G_a$  values shown in the SDPWS for the 6d common nails.

#### A3.2 STAPLES:

The staple deformation values shown in Table B must be used to determine diaphragm deflection in accordance with the IBC Section 2305.2 or shear wall deflection in accordance with the IBC Section 2305.3, as applicable.

Staple Gage	1	6	1	5	1	4			
Length (Inches)	1 <sup>1</sup> / <sub>2</sub>	2	1 <sup>3</sup> / <sub>4</sub>	<b>2</b> <sup>1</sup> / <sub>2</sub>	2	<b>2</b> <sup>1</sup> / <sub>2</sub>			
Load Per Fastener <sup>2</sup> (Pounds)		Connection Deflection <sup>3</sup> (Inches)							
60	0.008	0.003	0.008	0.005	0.005	0.003			
80	0.016	0.006	0.016	0.010	0.011	0.006			
100	0.032	0.008	0.028	0.015	0.019	0.009			
120	0.055	0.010	0.048	0.025	0.032	0.014			
140	0.087	0.024	0.077	0.040	0.050	0.021			
160	0.135	0.037	0.118	0.060	0.077	0.031			
180	0.205	0.052	0.173	0.088	0.113	0.044			
200	—	0.092	0.244	0.127	0.157	0.060			
220	—	0.198	0.299	0.178	0.219	0.080			
240	—	—	0.346	0.220	0.287	0.097			

# TABLE B—STAPLE DEFORMATION VALUES, en, FOR USE IN HORIZONTAL DIAPHRAGM AMD SHEAR WALL DEFLECTION ANALYSIS<sup>1,4</sup>

For **SI:** 1 inch = 25.4 mm 1 lbf = 4.45 N.

<sup>1</sup>Increase deformation value by 20% for plywood grades other than Structural I sheathing.

<sup>2</sup>Load per fastener is the diaphragm's maximum shear per foot divided by the number of fasteners per foot at interior panel edges.

<sup>3</sup>Values must be doubled for unseasoned lumber.

<sup>4</sup>Values are for  $e_n$  in equations found in the IBC.

#### APPENDIX B

#### **RECOGNIZED FASTENERS BY LISTEE**

#### PAGE NO. FOR THE TABLE WITH LISTEE NAME, PRODUCT BRAND NAME(S), AND LISTEE ADDRESS LISTEE SPECIFIC PRODUCT DESCRIPTIONS American Fasteners Company Ltd. (American Fasteners Brand) 11175 Inland Avenue 31 Jurupa Valley, California 91752 Beck America, Inc. (Fasco, Fasco/Beck, Beck Fastener Group Brands) 105 Industrial Park Drive 33 Muscle Shoals, Alabama 35661 Building Material Distributors, Inc. (Master Fasteners Brand) 225 Elm Avenue 31 Galt, California 95632 Falcon Fasteners Reg'd 251 Nantucket Boulevard 32 Toronto, Ontario M1P 2P2 CANADA Huttig Building Products (Huttig-Grip Brand) 555 Maryville University Drive, Suite 400 34 St. Louis, Missouri 63141 JAACO Corporation (NailPro Brand) 18080 NE 68th Street, Suite C130 34 Redmond, Washington 98052 Koki Holdings America Ltd. (Metabo HPT Brand) 1111 Broadway Avenue 35 Braselton, Georgia 30517 KYOCERA SENCO Industrial Tools, Inc. (Senco Brand) 4270 Ivy Pointe Boulevard 36 Cincinnati, Ohio 45245 Mid-Continent Steel and Wire (Mid-Continent Nail) (Magnum Fasteners Brand) 2700 Central Avenue 37 Poplar Bluff, Missouri 63901 Oman Fasteners, LLC. Plot No. 5117, Sohar PEIE Industrial Area - Phase 5, PO Box 584 37 Sohar, Sultanate of Oman, PC322 Paslode, an Illinois Tool Works Company (Paslode, Lightning Strike, Duo-Fast Brands) 155 Harlem Avenue 38 Glenview, Illinois 60025 Peace Industries (Spotnails Brand) 1100 Hicks Road 38 Rolling Meadows, Illinois 60008 PrimeSource Building Products (Grip-Rite, Fasteners Unlimited, Fits Rite Brands) 1321 Greenway Drive 39 Irving, Texas 75038 Specialty Fastening Systems, Inc. (Specialty Fasteners Brand) 424 South Baggett 39 Prairie Grove, Arkansas 72753 Stanley Black and Decker Inc. (BOSTITCH, DEWALT, Craftsman Brands) 701 East Joppa Road 40 Towson, Maryland 21286

#### TABLE B1—LISTEE INFORMATION AND INDEX TO LISTEE PRODUCT DESCRIPTIONS

#### **General Notes for Appendix B:**

- 1. For SI: 1 inch = 25.4 mm, 1 psi = 6.89 kPa.
- For each listee, nails having the diameter, shank type and finish type indicated in the applicable table are
  recognized for any length. Staples having the indicated diameter and finish are recognized for any leg
  length greater than 1<sup>1</sup>/<sub>2</sub> inches.
- 3. All nails are formed from carbon steel wire, unless designated in the tables below as stainless steel.
- 4. For a depiction of the various head styles, see Figure 1.

#### **Terminology**

LFRA = Lateral force resisting assembly: A diaphragm, shear wall or braced wall.

#### **Head Area Ratio Requirements**

- Y = 6d= Meets the head requirements for use in sheathing attachment in LFRAs where a 6d common nail is prescribed in the code. Y =8d = Meets the head requirements for use in sheathing attachment in LFRAs where a 8d common nail is prescribed in the code. Y =10d Meets the head requirements for use in sheathing attachment in LFRAs where a 10d common nail is prescribed in the code. = Meets the head requirements for use in sheathing attachment in LFRAs where a 16d common nail is prescribed in the code. Y =16d = Meets the head requirements for a 0.### diameter nail for use in sheathing attachment in shear walls and diaphragms in Y ### = accordance with Tables 5 through 8 of this report.
- N = Not recognized for use in sheathing attachment in LFRAs.
- n/a = Nail size is not prescribed in the code or listed in the diaphragm tables in this report for use in LFRAs. Use of Metal Hardware Nails in LFRAs is outside the scope of this report.

#### Shank Type

- S = Smooth shank nail
- R = Ring shank nail
- Sc = Screw shank nail

#### Finish/ Coating Types

- X = The fasteners are carbon steel, "bright" (ungalvanized, uncoated).
- H = Hardened (Bright) = Bending yield strength complies with Table S1.2 of ASTM F1667.
- HT = Bright, heat treated or hardened nail (may be the full nail or only a portion of the nail, such as the tip; compliance with Table S1.2 of F1667 has not been evaluated)
- SS = Stainless Steel
- HDG = Hot-dipped galvanized, complying with ASTM A153 Class D.
- EG = Electrogalvanized, complying with ASTM A641, Class 1.
- HHDG = Hardened and hot dip galvanized (Bending yield strength complies with Table S1.2 of ASTM F1667; galvanization complies with ASTM A153 Class D.)
- HEG = Hardened and electrogalvanized (Bending yield strength complies with Table S1.2 of ASTM F1667; galvanization complies with ASTM A641 Class 1.)
- HTHDG = Heat Treated or hardened and hot dip galvanized (Hardening may affect the full nail or only a portion of the nail, such as the tip; compliance with Table S1.2 of F1667 has not been evaluated. Galvanization complies with ASTM A153 Class D.)
- PP = Proprietary polymer coating applied to nail shank to aid in driving collated nails. May be applied over ungalvanized, galvanized or stainless steel nails.
- P# = Denotes a proprietary coating recognized in an ICC-ES evaluation report, as follows:
  - P1 = ThickCoat<sup>™</sup> recognized in ESR-1482.

			AMER	CAN FASTENERS CO	MPANY LT	D.						
	NAILS (Brand name: AMERICAN FASTENERS)											
TYPE OF COLLATION	NOMINAL DIAMETER (inch)	HEAD STYLE	RECOGNIZED FOR USE IN FRAMING	MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAS	SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIELD STRENGTH F <sub>yb</sub> (psi)				
	0.131		Y	Y =8d	S	X, HDG	Y	100,000				
Plastic, paper	0.135	Full round	Y	Y 135	S	X, HDG	Y	100,000				
	0.148		Y	Y =10d	S	X, HDG	Y	90,000				
	Metal Hardware Nails Designated as "Metal Connector Nail" on package labeling											
Deper	0.131	Full round	Y	n/a	S	X, HDG	Y	100,000				
Paper	0.148	Full round	Y	n/a	S	X, HDG	Y	90,000				

			BUILD	ING MATERIAL DISTRI	BUTORS, II	NC.		
			NAILS	(Brand name: MASTER	R FASTENE	RS)		
TYPE OF COLLATION	NOMINAL DIAMETER (inch)	HEAD STYLE	RECOGNIZED FOR USE IN FRAMING	MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAS	SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIELD STRENGTH F <sub>yb</sub> (psi)
	0.113		Y	Y =6d	S	X, HDG, EG	Y	100,000
	0.115		T	f –60	R	X, EG	Y	100,000
	0.120		Y	Y 120	S, Sc	X	Y	100,000
	0.120		T	1 120	R	X, EG	Y	100,000
Wire, plastic, paper	0.131	Full Round	Y	Y =8d	S	X	Y	100,000
paper	0.131			r –ou	R	X, EG	Y	100,000
	0.135		Y	n/a	S	Х	Y	100,000
	0.148		Y	Y =10d	S	X, HDG	Y	90,000
	0.140		Ť	r = 100	R	X	Y	90,000
Plastic	0.162		Y	Y =16d	S	X, HDG	Y	90,000
	0.113		Y	Y =6d	S	X, HDG, EG	Y	100.000
	0.115		Ť	r =ou	R	X, EG	Y	100,000
Dapar	0.120	Clippod	Y	Y 120	S, Sc	X	Y	100.000
Paper	0.120	Clipped	r	1 120	R	X, EG	Y	100,000
	0.131		Y	Y =8d	S	X, HDG, EG	Y	100,000
	0.131		r	r –ou	R	X, EG	Y	100,000

			•	FALCON FASTENERS NAILS										
TYPE OF COLLATION	NOMINAL DIAMETER (inch)	HEAD STYLE	RECOGNIZED FOR USE IN FRAMING	MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAS	SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIEL STRENGTH F <sub>yb</sub> (psi)						
	0.092		Y*	n/a	S, R, Sc	Х	Y	100,000						
Wire					S	X, HDG, SS, HT, HTHDG	Y							
wire	0.099		Y	n/a	R	X, HDG, SS	Y	100,000						
					Sc	X, HDG	Y							
					S	X, HDG, SS, HT, HTHDG	Y							
	0.113		Y	Y =6d	R	X, HDG, SS	Y	100,000						
					Sc	X, HDG	Y	BENDING YIELD STRENGTH <i>F<sub>yb</sub></i> (psi) 100,000 100,000						
		Full round			S	X, HDG, SS, HT, HTHDG	Y							
	0.120	i un round	Y	Y 120	R	X, HDG, SS	Y	BENDING YIEI STRENGTH Fyb (psi)           100,000           100,000           100,000           100,000           100,000           90,000           90,000           100,000           100,000           100,000           100,000           100,000           100,000           100,000           100,000           100,000           100,000           100,000           100,000           100,000           100,000           100,000           100,000           100,000           100,000           90,000           90,000           100,000						
Wire, plastic,					Sc	X, HDG	Y							
paper					S	X, HDG, SS, HT, HTHDG	Y							
	0.131		Y	Y =8d	R	X, HDG, SS	Y	100,000						
					Sc	X, HDG	Y							
	0.148		Y	Y =10d	S	X, HDG, SS, HT, HTHDG	Y	90,000						
	0.140		Т	f – 100	R	X, HDG, SS	Y	90,000						
	0.162		Y	Y =16d	S	X, HDG, HT, HTHDG	Y	90,000						
					S	X, HDG, SS, HT, HTHDG	Y							
	0.113		Y	Y =6d	R	X, HDG, SS	Y	100,000						
					Sc	X, HDG	Y							
Paper, wire		Notched				S	X, HDG, SS, HT, HTHDG	Y						
	0.120		Y	Y 120	R	X, HDG, SS	Y	100,000						
					Sc	X, HDG	Y							
					S	X, HDG, SS, HT, HTHDG	Y							
	0.131		Y	Y =8d	R	X, HDG, SS	Y	100 000						
	0.101			1 -00	Sc	X, HDG	Y	100,000						
					S	X, HDG, SS, HT, HTHDG	Y							
	0.113		Y	Y =6d	R	X, HDG, SS	Y	100.000						
	0.115		I I	1 -00	Sc	X, HDG, 33	Y	100,000						
					S	X, HDG, SS, HT, HTHDG	Y							
	0.400		Y	V 400	R		Y Y	100.000						
Dener	0.120	Clipped	Clinned	0			Oliveral		T	Y 120	Sc	X, HDG, SS X, HDG	Y	100,000
Paper		Clipped				,								
	0.404			N OI	S	X, HDG, SS, HT, HTHDG	Y	100.000						
	0.131		Y	Y =8d	R	X, HDG, SS	Y	100,000						
					Sc	X, HDG	Y							
	0.148		Y	Y =10d	S	X, HDG, SS, HT, HTHDG	Y	90,000						
					R	X, HDG, SS	Y							
					S	X, HDG, SS, HT, HTHDG	Y							
	0.113		Y	Y =6d	R	X, HDG, SS	Y	100,000						
					Sc	X, HDG	Y							
					S	X, HDG, SS, HT, HTHDG	Y							
Paper, wire	0.120		Y	Y 120	R	X, HDG, SS	Y	100,000						
		Offset			Sc	X, HDG	Y							
					S	X, HDG, SS, HT, HTHDG	Y							
	0.131		Y	Y =8d	R	X, HDG, SS	Y	100,000						
					Sc	X, HDG	Y							
Paper	0.148		Y	Y =10d	S	X, HDG, SS, HT, HTHDG	Y	90,000						
					R	X, HDG, SS	Y	50,000						
	Meta	I Hardware N	ails - Designated	as "Metal Connector Nail'	' or "Joist H	langer Nail" on package la	beling							
	0.131		Y		S	X, HDG, SS, HT, HTHDG	Y	100 000						
	0.101				R	X, HDG, SS	Y	100,000						
Paper	0.148	Full round**	Y	n/a	S	X, HDG, SS, HT, HTHDG	Y	90.000						
	-				R	X, HDG, SS	Y							
	0.162		Y		S	X, HDG, HT, HTHDG	Y	00,000						

			(Brand names)	BECK AMERICA,					
TYPE OF COLLATION	NOMINAL DIAMETER (inch)	HEAD	RECOGNIZED FOR USE IN FRAMING	EFASCO, FASCO/BEC MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAS	SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIELD STRENGTH Fyb (psi)	
	0.092		Y*	n/a	S, R	X, HDG,EG, SS	Y	100,000	
	0.092	Full round Clipped Offset	1	11/a	Sc	X, HDG, EG	Y	100,000	
	0.099		Y	n/a	S, R	X, HDG, EG, SS	Y	100.000	
	0.099		T	n/a	Sc	X, HDG, EG	Y	100,000	
	0.113		Y	Y =6d	S, Sc	X, HDG, EG	Y	100,000	
Wire, plastic,	0.115		I	1 -00	R	X, HDG, EG, SS	Y	100,000	
paper	0.120		Y	Y 120	S	X, HDG, EG	Y	100,000	
	0.120			1 120	R, Sc	X, HDG, EG, SS	Y	100,000	
	0.131	Full round	Y	Y =8d	S	X, HDG, EG	Y	100,000	
	0.101				R, Sc	Х	Y	100,000	
	0.135		Y	N	S, R, Sc	Х	Y	100,000	
	0.148		Y	Y =10d	S, R	X, HDG, EG	Y	90,000	
					Sc	X, HDG	Y	,	
Wire, plastic	0.162		Y	N	S, Sc	X, HDG	Y	90,000	
		_			R	X	Y		
Plastic	0.180		Y	n/a	S	X	Y	80,000	
	0.197		Y	n/a	S	X	Y	80,000	
	0.092		Y*	n/a	S, R	X, HDG, EG, SS	Y Y	100,000	
		Clipped			Sc	X, HDG, EG	Y Y		
	0.113		Y	Y =6d	S, Sc R	X, HDG, EG	Y Y	100,000	
Wire, plastic,					R S	X, HDG, EG, SS X, HDG, EG	ř Y		
paper	0.120		Y	Y 120	R, Sc	X, HDG, EG, SS	Y	100,000	
							S S	X, HDG, EG	Y
	0.131		Y	N	R, Sc	X, 1120, 20	Ý	100,000	
	0.135		Y	N	S, R, Sc	X	Ý	100,000	
	0.100				S, Sc	X, HDG, EG	Y		
	0.113		Y	Y =6d	R	X, HDG, EG, SS	Y	100,000	
Wire, plastic,		Offset			S	X, HDG, EG	Y		
paper	0.120		Y	N	R, Sc	X, HDG, EG, SS	Y	100,000	
	0.135		Y	N	S, R, Sc	Х	Y	100,000	
			Designated "Pap	Metal Hardware N per Tape joist Hanger N	ails ails" on pack	age labeling			
	0.131		Y	n/a	S	X, HDG, EG	Y	100,000	
Paper	0.148	Full Round	Y	n/a	S	X, HDG, EG	Y	90,000	
	0.162		Y	n/a	S	X, HDG, EG	Y	90,000	
		STAPLE	S (Brand name	es: FASCO, FASCO/BE	-				
	GAGE			NOMINAL CROWN	VIDTH (inch	1)	FINISH/ C		
	16			<sup>7</sup> / <sub>16</sub>			X, E		
	15			<sup>7</sup> / <sub>16</sub>			X, E		
	14		tions subjected	1			X, E	ĒG	

			NA	ILS (Brand name: HU)	TIG-GRIP)			
TYPE OF COLLATION	NOMINAL DIAMETER (inch)	HEAD STYLE	RECOGNIZED FOR USE IN FRAMING	MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAS	SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIELD STRENGTH Fyb (psi)
Wire	0.092		Y*	n/a	S, R, Sc	X, HDG, EG	Y	100,000
wire	0.099		Y	n/a	S, R, Sc	X, HDG, EG	Y	100,000
	0.113		Y	Y =6d	S, R, Sc	X, HDG, EG	Y	100,000
Wire, Plastic	0.120	Full round	Y	Y 120	S, R, Sc	X, HDG, EG	Y	100,000
	0.131		Y	Y =8d	S, R, Sc	X, HDG, EG	Y	100,000
	0.135		Y	Y 135	S	Х	Y	100,000
Plastic	0.148	-	Y	Y =10d	S, R, Sc	X, HDG, EG	Y	90,000
	0.162		Y	Y =16d	S, R, Sc	X, HDG, EG	Y	90,000
	0.113	Clipped	Y	Y =6d	S, R, Sc	X, HDG, EG	Y	100,000
	0.120		Y	Y 120	S, R, Sc	X, HDG, EG	Y	100,000
Daway	0.131		Y	Y =8d	S, R, Sc	X, HDG, EG	Y	100,000
Paper	0.113		Y	Y =6d	S, R, Sc	X, HDG, EG	Y	100,000
	0.120	Offset	Y	Y 120	S, R, Sc	X, HDG, EG	Y	100,000
	0.131		Y	Y =8d	S, R, Sc	X, HDG, EG	Y	100,000
				Metal Hardware N				
			` <b>`</b>	Metal Connector Nail"		0,		
	0.131	Full	Y	n/a	S	H, HHDG, HEG	Y	130,000
Paper	0.148	Round**	Y	n/a	S	H, HHDG, HEG	Y	115,000
	0.162		Y	n/a	S	H, HHDG, HEG	Y	115,000

'1' for  $1^{1}_{2} \times 0.131$ ; '2' for  $2^{1}_{2} \times 0.131$ ; '3' for  $1^{1}_{2} \times 0.148$ ; '4' for  $2^{1}_{2} \times 0.148$ ; '5' for  $2^{1}_{2} \times 0.162$ 

	JAACO CORPORATION										
	NAILS (Brand name: NAILPRO)										
TYPE OF COLLATION	NOMINAL DIAMETER (inch)	HEAD STYLE	RECOGNIZED FOR USE IN FRAMING	MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAS	SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIELD STRENGTH <i>F<sub>yb</sub></i> (psi)			
	0.113		Y	Y =6d	S	Х	Y	100,000			
Wire, plastic	0.120		Y	Y 120	S, R	Х	Y	100,000			
Wire, plastic, paper	0.131	Full Round	Y	Y =8d	S	x	Y	100,000			
Dapar	0.148		Y	Ν	S	Х	Y	90,000			
Paper	0.162		Y	Ν	S	Х	Y	90,000			
Metal Hardware Nails Designated "Joist Hanger Nail" on package labeling											
	0.131		Y	n/a	S	Х	N	100,000			
Paper	0.148	Full Round	Y	n/a	S	Х	N	90,000			
	0.162		Y	n/a	S	Х	Ν	90,000			

			NA	ILS (Brand name: ME1	(ABO-HPT)			
TYPE OF COLLATION	NOMINAL DIAMETER (inch)	HEAD STYLE	RECOGNIZED FOR USE IN FRAMING	MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAS	SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIELD STRENGTH Fyb (psi)
	0.000			,	S,R,Sc	X, EG, HDG, SS	Y	400.000
	0.092		Y*	n/a	Sc	X, EG, HDG	Y	100,000
	0.099		Y	n/a	S, R, Sc	X, EG, HDG	Y	100,000
Γ					S	X, EG, HDG, SS, HT	Y	
	0.113		Y	Y =6d	R	X, EG, HDG, SS	Y	100,000
					Sc	X, EG, HDG	Y	
Wire, plastic, paper					S	X, EG, HDG, SS, HT	Y	100,000
	0.120	Full round	Y	Y 120	R	X, EG, HDG, SS	Y	100,000
		i un round			Sc	X, EG, HDG	Y	90,000
		0 131	N N	Y =8d	S	X, EG, HDG, SS, HT	Y	100.000
	0.131		Y	Y =8d	R	X, EG, HDG, SS	Y	100,000
					Sc	X, HDG	Y	<i>F<sub>yb</sub></i> (psi) 100,000 100,000 100,000 100,000 100,000
	0.440		X	V 404	S	X, EG, HDG, SS, HT	Y	00.000
	0.148		Y	Y =10d	R	X, EG, HDG, SS	Y	90,000
					Sc	X, HDG	Y	
	0.162		Y	Ν	S	X, EG, HDG	Y	90,000
	0.113		Y	Y =6d	S, R, Sc	X, EG, HDG	Y	100,000
Paper	0.120	Clipped	Y	Y 120	S, R, Sc	X, EG, HDG	Y	100,000
	0.131		Y	Y =8d	S, R, Sc	X, EG, HDG	Y	100,000
	0.113		Y	Y =6d	S, R, Sc	X, EG, HDG	Y	100,000
Wire, paper	0.120	Offset	Y	Y 120	S, R, Sc	X, EG, HDG	Y	100,000
	0.131		Y	Y =8d	S, R, Sc	X, EG, HDG	Y	100,000
			Designa	Metal Hardware N ated "Strap-Tite" on pa		ling		
	0.131		Y	n/a	S	X, HHDG, HEG	Y	130,000
Paper	0.148	Full round	Y	n/a	S	X, HHDG, HEG	Y	115,000
	0.162		Y	n/a	S	X, HHDG, HEG	Y	115,000
Recognition i	s limited to u	se in connec	tions subjected	to tension only.				

			KYOCE	RA SENCO INDUSTRIA	AL TOOLS,	INC.								
				NAILS (Brand name: S	SENCO)									
TYPE OF COLLATION	NOMINAL DIAMETER (inch)	HEAD STYLE	RECOGNIZED FOR USE IN FRAMING	MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAS	SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIELD STRENGTH <i>F<sub>yb</sub></i> (psi)						
	0.113		Y	Y =6d	S, R	X, EG, HDG	Y	100,000						
Plastic, wire	0.120		Y	Y 120	S, Sc	X, EG, HDG	Y	100,000						
	0.120		I	1 120	R	X, HDG	Y	100,000						
	0.131		Y	Y =8d	S	X, EG, HDG	Y	100.000						
Plastic, paper, wire	0.131	Full Round	I	1 –ou	R	X, HDG	Y	100,000						
paper, me	0.135		Y	Y 135	S	Х	Y	100,000						
Plastic, paper	0.148		Y	Y =10d	S	X, HDG	Y	90,000						
Paper	0.162		Y	Y =16d	S	X, HDG, HT	Y	90,000						
	0.113	_	Y	Y =6d	S, R	X, EG, HDG	Y	100,000						
	0.120		Y	Y 120	S	X, EG, HDG	Y	100,000						
Paper	0.120	Clipped	T	1 120	R	X, HDG	Y	100,000						
Faper	0.131		Y	Y =8d	S	X, EG, HDG	Y	100.000						
	0.131						-	-	I	1 –ou	R	X, HDG	Y	100,000
	0.135		Y	Y 135	S	Х	Y	100,000						
	0.113		Y	Y =6d	S, R	X, HDG	Y	100,000						
Paper	0.120	Offset	Y	Y 120	S, R	X, HDG	Y	100,000						
	0.131		Y	Y =8d	S, R	X, HDG	Y	100,000						
			(Designated "Har	Metal Hardware Na dened Metal Connector N		kage labeling)								
	0.131		Y	n/a	S	H, HHDG, HEG	Y	130,000						
Paper	0.148	Full Round	Y	n/a	S	H, HHDG, HEG	Y	115,000						
	0.162	1	Y	n/a	S	H, HHDG, HEG	Y	115,000						
			S	TAPLES (Brand name:	SENCO)			•						
	GAGE			IAL CROWN WIDTH (in	ich)	FIN	SH/ COATI	NG						
	14			<sup>7</sup> / <sub>16</sub>			Х							
	15			7/ <sub>16</sub>			X, EG							
	16			<sup>7</sup> / <sub>16</sub> , 1			X, EG							

			MID-CONTINEN	IT STEEL AND WIRE (M	ID-CONTIN	IENT NAIL)				
			NAILS (	Brand name: MAGNUM	I FASTENE	RS)				
TYPE OF COLLATION	NOMINAL DIAMETER (inch)	HEAD STYLE	RECOGNIZED FOR USE IN FRAMING	MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAS	SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIELD STRENGTH F <sub>yb</sub> (psi)		
Wire	0.099		Y	n/a	S,R,Sc	X, EG, HDG	Y	100,000		
	0.113		Y	Y =6d	S,R,Sc	X, EG, HDG	Y	100,000		
Wire, paper, plastic	0.120		Y	Y 120	S,R,Sc	X, EG, HDG	Y	100,000		
plactic	0.131	Full round	Y	Y =8d	S,R,Sc	X, EG, HDG	Y	100,000		
Wire, plastic	0.135		Y	Y 135	S,R,Sc	X, EG, HDG	Y	100,000		
Diastia	0.148		Y	Y =10d	S,R,Sc	X, EG, HDG	Y	90,000		
Plastic	0.162		Y	Ν	S,R,Sc	X, EG, HDG	Y	90,000		
	0.113		Y	Ν	S,R,Sc	X, HDG	Y	100,000		
Paper	0.120	Offset	Y	Ν	S,R,Sc	X, HDG	Y	100,000		
	0.131		Y	Ν	S,R,Sc	X, HDG	Y	100,000		
Metal Hardware Nails Designated "Hardware Nails" on package labeling										
Denen	0.131		Y	n/a	S	X, HDG	Y*	100,000		
Paper, plastic	0.148	Full round	Y	n/a	S	X, HDG	Y*	90,000		
P.0000	0.162		Y	n/a	S	X, HDG	Y*	90,000		
*Proprietary p	olymer coatir	ng added to 2	2 <sup>1</sup> / <sub>2</sub> " length nails	only.						

TYPE OF	NOMINAL DIAMETER (inch)	HEAD STYLE	RECOGNIZED FOR USE IN FRAMING	NAILS MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAS	SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIELD STRENGTH <i>F<sub>yb</sub></i> (psi)
Wire	0.092		Y*	n/a	S, R, Sc	X, HDG, EG	Y	100,000
wire	0.099		Y	n/a	S, R, Sc	X, HDG, EG	Y	100,000
	0.113		Y	Y =6d	S, R, Sc	X, HDG, EG	Y	100,000
Wire, plastic	0.120	Full Round	Y	Y 120	S, R, Sc	X, HDG, EG	Y	100,000
	0.131		Y	Y =8d	S, R, Sc	X, HDG, EG	Y	100,000
Plastic	0.148		Y	Y =10d	S, R, Sc	X, HDG, EG	Y	90,000
Flastic	0.162		Y	Y =16d	S, R, Sc	X, HDG, EG	Y	90,000
	0.113		Y	Y =6d	S, R, Sc	X, HDG, EG	Y	100,000
	0.120	Clipped	Y	Y 120	S, R, Sc	X, HDG, EG	Y	100,000
_ [	0.131		Y	Y =8d	S, R, Sc	X, HDG, EG	Y	100,000
Paper	0.113	Offset	Y	Y =6d	S, R, Sc	X, HDG, EG	Y	100,000
	0.120		Y	Y 120	S, R, Sc	X, HDG, EG	Y	100,000
	0.131		Y	Y =8d	S, R, Sc	X, HDG, EG	Y	100,000
		(D	esignated "Ha	Metal Hardware Na dened Joist Hanger Na		age labeling)		
	0.131		Y	n/a	S	H, HHDG, HEG	Y	130,000
Paper	0.148	Full Round**	Y	n/a	S	H, HHDG, HEG	Y	115,000
	0.162	Round	Y	n/a	S	H, HHDG, HEG	Y	115,000
Recognition is	s limited to u	se in connec	tions subjected	to tension only.				

			PASLODE, AN I	LLINOIS TOOL WORK	(S COMPA	NY		
		NAILS (I	Brand names: P	ASLODE, LIGHTNING	STRIKE, D	UO-FAST)		
TYPE OF COLLATION	NOMINAL DIAMETER (inch)	HEAD STYLE (Applicable Brand Name)	RECOGNIZED FOR USE IN FRAMING	MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAS	SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIELD STRENGTH F <sub>yb</sub> (psi)
	0.113	Full Round	Y	Y =6d	S, R	X, HDG	Y	100,000
Plastic,	0.120		Y	Y 120	R, Sc	HDG	Y	100,000
paper	0.131	Full Round	Y	Y =8d	S	X, HDG	Y	100.000
			T	r –ou	Sc	HDG	Y	100,000
	0.148		Y	Y =10d	S	Х	Y	90,000
	0.113		Y	Y =6d	S, R	X, HDG	Y	100,000
Paper	0.120	Offset (RounDrive™)	Y	Y 120	S, R	х	Y	100,000
	0.135	(RounDrive™)	Y	Y 135	S	х	Y	100,000
	0.113	Clipped (Paslode)	Y	Y =8d	S, R	HDG	Y	100,000
Paper	0.120		Y	Y 120	S, R, Sc	X, HDG	Y	100,000
	0.131	(1 201000)	Y	Ν	S, R	Х	Y	100,000

				PEACE INDUSTRIES				
			NAIL	S (Brand name: SPOT	NAILS)			
TYPE OF COLLATION	NOMINAL DIAMETER (inch)	HEAD STYLE	RECOGNIZED FOR USE IN FRAMING	MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAS	SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIELD STRENGTH Fyb (psi)
					S	X, EG	Y	100,000
	0.099		Y	n/a	R	EG	Y	100,000
		-			Sc	х	Y	100,000
Wire	0.113		Y	Y =6d	S, R	Х	Y	100,000
	0.400	Full Round	Y		S	X, EG	Y	100,000
	0.120			Y 120	R, Sc	Х	Y	100,000
	0.131		Y	Y =8d	S	Х	Y	100,000
	0.440		X	)	S, Sc	Х	Y	100,000
	0.113		Y	Y =6d	R	X, EG	Y	100,000
Plastic	0.120		Y	Y 120	S, Sc	Х	Y	100,000
	0.120		Ť	¥ 120	R	EG	Y	100,000
	0.131		Y	Y =8d	S	Х	Y	100,000
	0.113		Y	Y =6d	R	Х	Y	100,000
Paper	0.120	Clipped	Y	Y 120	R	HDG	Y	100,000
	0.131		Y	Y =8d	S	Х	Y	100,000
			(Designated	Metal Hardware Nails "Joist Hanger Nail" on pa	ckage labeling	3)		
	0.131		Y	n/a	S	H, HHDG, HEG	Y	130,000
Paper	0.148	Full Round	Y	n/a	S	H, HHDG, HEG	Y	115,000
1	0.162		Y	n/a	S	H, HHDG, HEG	Y	115,000

			PRIMES	SOURCE BUILDING PR	ODUCTS						
	NAILS (Brand names: GRIP-RITE, FAS'NERS UNLIMITED, FITS RITE)										
TYPE OF COLLATION	NOMINAL DIAMETER (inch)	HEAD STYLE	RECOGNIZED FOR USE IN FRAMING	MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAS	SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIELD STRENGTH Fyb (psi)			
	0.113		Y	Y =6d	S, R, Sc	Х	Y	100,000			
Wire	0.120	Full Round	Y	Y 120	S, R, Sc	Х	Y	100,000			
	0.131		Y	Y =8d	S, R, Sc	Х	Y	100,000			
	0.135		Y	Y 135	S	х	Y	100,000			
Plastic	0.148	Full Round	Y	Y =10d	S, R, Sc	Х	Y	90,000			
	0.162		Y	Y =16d	S, R, Sc	Х	Y	90,000			
	0.113		Y	Y =6d	S, R	Х	Y	100,000			
Wire, paper	0.120	D head (clipped)	Y	Y 120	S, R	Х	Y	100,000			
	0.131	(onpped)	Y	Y =8d	S, R	Х	Y	100,000			
	0.113	Offset	Y	Y =6d	S	Х	Y	100,000			
Wire, paper	0.120		Y	Y 120	S	Х	Y	100,000			
	0.131		Y	Y =8d	S	Х	Y	100,000			

				ALTY FASTENING SYS				
TYPE OF COLLATION	NOMINAL DIAMETER (inch)	HEAD STYLE	(Brand names: RECOGNIZED FOR USE IN FRAMING	SPECIALTY NAIL CO. MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAS	, SPECIALT SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIELD STRENGTH F <sub>yb</sub> (psi)
	0.092		Y*	n/a	S,R,Sc	Х	Ν	100,000
					S	X, HDG, SS, HT	Ν	100,000
	0.099		Y	n/a	R	X, HDG, SS	Ν	
					Sc	X, HDG	Ν	
-					S	X, HDG, SS, HT	Ν	100,000
	0.113		Y	Y =6d	R	X, HDG, SS	Ν	
					Sc	X, HDG	Ν	
Plastic,		Full round			S	X, HDG, SS, HT	N	100,000
paper	0.120	Full round	Y	Y 120	R	X, HDG, SS	Ν	
					Sc	X, HDG	Ν	Fyb (psi)           100,000           100,000           100,000           100,000           100,000           100,000           90,000
					S	X, HDG, SS, HT	N	100,000
	0.131		Y	Y =8d	R	X, HDG, SS	Ν	
					Sc	X, HDG	Ν	
	0.148			Y =10d	S	X, HDG, SS, HT	Ν	90,000
	0.140		Y	1 - 100	R	X, HDG, SS	Ν	
	0.162		Y	Y =16d	S	X, HDG, HT	Ν	90,000
'Recognition i	s limited to us	e in connect	ions subjected to	tension only.				

		NAI		BLACK AND DECKER I BOSTITCH, DEWALT, (		AN)			
TYPE OF COLLATION	NOMINAL DIAMETER (inch)	HEAD STYLE	RECOGNIZED FOR USE IN FRAMING	MEETS HEAD AREA RATIO REQUIREMENTS FOR USE IN LFRAs	SHANK TYPE	FINISH/ COATING	PP COATING	SPECIFIED BENDING YIE STRENGTH Fyb (psi)	
Wire, plastic	0.092	Full Round	Y	n/a	S, R	X, HDG	Y	100,000	
					Sc	X Y	100,000		
	0.099		Y	n/a	S, R, Sc	X, HDG, P1(B) <sup>1</sup>	Y	100,000	
	0.113		Y	Y =6d	S, R	X, HDG, EG, P1	Y	100,000	
			Y	Y =6d	Sc	X, HDG, P1(B)	Y		
	0.120		Y	Y 120	S, R	X, HDG, EG, P1	Y	100,000	
			Y	Y 120	Sc	X, HDG, P1(B)	Y		
	0.131		Y	Y =8d	S	X, HDG, EG, P1	Y	100,000	
			Y	Y =8d	R	X, HDG, EG	Y		
			Y	Y =8d	Sc	X, HDG, P1(B)	Y		
	0.135		Y	Y 135	S	х	Y	100,000	
	0.148		Y	Y =10d	S, Sc	х	Y	90,000	
	0.162		Y	N	S	X, P1(B)	Y	90,000	
Paper	0.113	Clipped	Y	Y =6d	S, R	X, HDG, EG, P1	Y	100,000	
			Y	Y =6d	Sc	X, HDG, P1	Y		
	0.120		Y	Y 120	S, R	X, HDG, EG, P1	Y	100,000	
			Y	Y 120	Sc	X, HDG, P1	Y		
	0.131		Y	Y =8d	S, R	X, HDG, EG, P1	Y	100,000	
			Y	Y =8d	Sc	X, HDG, P1	Y		
Wire, paper	0.113	Offset	Y	Y =6d	S, R	X, HDG, EG, P1	Y	100,000	
			Y	Y =6d	Sc	X, HDG, P1	Y		
	0.120		Y	Y 120	S, R	X, HDG, EG, P1	Y	100,000	
			Y	Y 120	Sc	X, HDG, P1	Y		
	0.131		Υ	Y =8d	S, R	X, HDG, EG, P1	Y	100,000	
			Y	Y =8d	Sc	X, HDG, P1	Y		
Wire	0.113	Notched	Y	Y =6d	S, R	X, HDG, EG, P1	Y	100,000	
			Y	Y =6d	Sc	X, HDG, P1	Y		
	0.120		Y	Y 120	S, R	X, HDG, EG, P1	Y	100,000	
			Y	Y 120	Sc	X, HDG, P1	Y		
	0.131		Y	Y =8d	S, R	X, HDG, EG, P1	Y	100,000	
			Y	Y =8d	Sc	X, HDG, P1	Y	1	
		п		etal Hardware Nails Connector Nail" on packa	ge labeling	**			
Paper, plastic	0.131	Full Round**	Y	n/a	S	X, P1, HT	Y	100,000	
	0.148		Y	n/a	S	X, P1, HT	Y	90,000	
	0.162		Y	n/a	S	X, P1, HT	Y	90,000	
When used, hea	d markings for m	netal hardwar		x 0.148; 'B5' for 2 <sup>1</sup> / <sub>2</sub> x 0.16					
			STAPLES (Brai	nd name: BOSTITCH, DEV	· · ·				
	<b>GAGE</b> 16			NOMINAL CROWN WIDTH (inch) 7/ <sub>16,</sub> 1/ <sub>2,</sub> 1				FINISH/COATING X, EG	

<sup>1</sup>P1(B) means that the P1 coating is only available under the BOSTITCH Brand Name.



### **ICC-ES Evaluation Report**

### ESR-1539 LABC and LARC Supplement

Reissued July 2018 Revised June 2019 This report is subject to renewal July 2020.

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A Subsidiary of the International Code Council®

DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES Section: 06 05 23.13—Nails Section: 06 05 23.15—Staples

#### **REPORT HOLDER:**

#### INTERNATIONAL STAPLE, NAIL AND TOOL ASSOCIATION (ISANTA)

#### **EVALUATION SUBJECT:**

#### **POWER-DRIVEN STAPLES AND NAILS**

#### 1.0 REPORT PURPOSE AND SCOPE

#### Purpose:

The purpose of this evaluation report supplement is to indicate that the Power-Driven Staples and Nails described in ICC-ES master evaluation report <u>ESR-1539</u>, have also been evaluated for compliance with the codes noted below as adopted by the Los Angeles Department of Building and Safety (LADBS).

#### Applicable code editions:

- 2017 City of Los Angeles Building Code (LABC)
- 2017 City of Los Angeles Residential Code (LARC)

#### 2.0 CONCLUSIONS

The Power-Driven Staples and Nails, described in Sections 2.0 through 7.0 of the master evaluation report <u>ESR-1539</u>, comply with the LABC Chapter 23 and the LARC and are subject to the conditions of use described in this supplement.

#### 3.0 CONDITIONS OF USE

The Power-Driven Staples and Nails described in this evaluation report must comply with all of the following conditions:

- All applicable sections in the master evaluation report <u>ESR-1539</u>.
- The design, installation, conditions of use and identification of the nails and staples are in accordance with the 2015 International Building Code<sup>®</sup> (2015 IBC) provisions noted in the master evaluation report <u>ESR-1539</u>.
- The design, installation and inspection are in accordance with additional requirements of LABC Chapters 16 and 17, and Sections 2304.10, 2305, 2306 and 2308, and LARC Sections R502, R503, R602, R802 and R803, as applicable.
- In accordance with LABC Sections 2306.2 and 2306.3, engineered diaphragms and shear walls constructed with staples as described in Section 4.1.2 of the master evaluation report <u>ESR-1539</u> are permitted only for structures assigned to Seismic Design Category A, B or C.
- Nails and staples made from bright steel wire must not be used in exterior or exposed conditions.
- The hillside building provisions in LABC Section 2301.1 are excluded from this supplement.

This supplement expires concurrently with the master report, reissued July 2018 and revised June 2019.





### **ICC-ES Evaluation Report**

### **ESR-1539 FBC Supplement**

Reissued July 2018 Revised June 2019 This report is subject to renewal July 2020.

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DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES Section: 06 05 23.13—Nails Section: 06 05 23.15—Staples

**REPORT HOLDER:** 

INTERNATIONAL STAPLE, NAIL AND TOOL ASSOCIATION (ISANTA)

#### **EVALUATION SUBJECT:**

#### **POWER-DRIVEN STAPLES AND NAILS**

#### 1.0 REPORT PURPOSE AND SCOPE

#### Purpose:

The purpose of this evaluation report supplement is to indicate that Power-Driven Staples and Nails recognized in ICC-ES master evaluation report ESR-1539, have also been evaluated for compliance with the codes noted below.

#### Applicable code editions:

- 2017 Florida Building Code—Building
- 2017 Florida Building Code—Residential

#### 2.0 CONCLUSIONS

The Power-Driven Staples and Nails, described in Sections 2.0 through 7.0 and Appendix B of the master evaluation report ESR-1539, comply with the *Florida Building Code—Building* and the *Florida Building Code—Residential*, provided the design and installation are in accordance with the 2015 *International Building Code*<sup>®</sup> (IBC) provisions noted in the master report under the following conditions:

- For stainless steel nails, the reference withdrawal design value must be determined in accordance with the 2018 ANSI/AWC National Design Specification (NDS).
- For nails shown as having round heads in Appendix B of the master report, reference head pull-through values must be determined in accordance with Section 12.2.5 of the 2018 NDS.

Use of the Power-Driven Staples and Nails has also been found to be in compliance with the High-Velocity Hurricane Zone provisions of the *Florida Building Code—Building* and the *Florida Building Code—Residential*.

For products falling under Florida Rule 9N-3, verification that the report holder's quality assurance program is audited by a quality assurance entity approved by the Florida Building Commission for the type of inspections being conducted is the responsibility of an approved validation entity (or the code official, when the report holder does not possess an approval by the Commission).

This supplement expires concurrently with the master report, reissued July 2018 and revised June 2019.

